

APPENDIX B: AVAILABLE DATA

Name of File	Type of Data	Date Received
Flood Study.pdf	Wagonga Inlet Flooding Investigation	27 April 2012
	by Gary Blumberg and Associates	·
Wagonga_Inlet_Estuary_	Wagonga Inlet Estuary Processes	28 May 2012
Processes Study.pdf	Study by MHL	
wagonga.xyz	Bathymetry of Wagonga Inlet	27 April 2012
53175SH1.pdf	Plan of Wagonga Inlet Hydrographic	27 April 2012
53.7.55.7.7. p a.	Survey	
ESC_lakes_REF&policies_final.pdf	Report of Eurobodalla Coastal Lakes	27 April 2012
	Entrance Management Policies –	
	Review of Environmental Factors	
Multiple JPG files	Photos of flooding in the 2010 event	3 May 2012
bridges_culverts.tab	Shape-file of points showing location	7 May 2012
	of bridges or culverts	1 may 2012
buildings.tab	Shape-file of polylines outlining	7 May 2012
buildings.tab	council owned buildings and land	7 May 2012
	(including ovals etc)	
cadastre.tab	Shape-file of property polygons	7 May 2012
contour_10m.tab	Ground contours	7 May 2012
roads.tab	Centre-lines of roads including the	7 May 2012
Toads.tab	road name as an attribute	7 May 2012
conn14 tob		7 May 2012
sepp14.tab	Shape-file of polygons of SEPP 14 Wetlands	7 May 2012
74 to b		7.14 0040
sepp71.tab	Shape-file of polygons of SEPP 71	7 May 2012
	Coastal Lakes	714, 0040
sewgpipe.tab	Pipes carrying sewer water via	7 May 2012
	gravity	
sewrpipe.tab	Pipes carrying sewer water via	7 May 2012
	pressure	-11 0010
sewnode.tab	Shape-file of points connected to	7 May 2012
	sewrpipe.tab and sewgpipe.tab	
sewtplant.tab	Shape-file of polygons outlining	7 May 2012
	treatment plant infrastructure, such	
	as drying beds and sludge ponds	
stwpipe.tab	Pipes carrying stormwater	7 May 2012
stwnode.tab	Shape-file of points connected to	7 May 2012
	stwpipe.tab	
watercourse.tab	Shape-file of polylines outlining the	7 May 2012
	shoreline of waterbodies (lakes etc)	
	and the centre-line of watercourses	
Narooma_2010 as TAB and ECW	Aerials from 2010	7 May 2012
files		
KE#### as TAB and ECW files	Aerials from 2006	7 May 2012
Multiple THN files	Raw LiDAR collected in 2006	7 May 2012
LSZ_20120316.shp	Eurobodalla LEP 2012 (specifying lot size)	10 May 2012
LZN_20120315.shp	Eurobodalla LEP 2011 (specifying	10 May 2012
LETT_20120010.511p	zones)	10 May 2012
Rain Gauge Data.csv	Continuous Pluviometer Rainfall	29 May 2012
3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 -	Data for Council owned stations at	
	Narooma and Tuross Head	

Bridge Data Wagonga and Dalmeny.xlsx	Data for Council owned bridges on Wagonga Scenic Drive and Dalmeny Drive	29 May 2012
Barlows Bay (Live).csv Bermagui (Live).csv Ulladulla Harbour (Live).csv	Data from Water Level Stations owned by MHL	25 June 2012
Barlows Bay Rain (Live).csv	Data from Continuous Pluviometer Stations owned by MHL	25 June 2012
HM01X_Data_069017.txt HM01X_Data_069148.txt	Data from Continuous Pluviometer Stations owned by BOM	6 July 2012
Multiple XYZ files	Raw LiDAR collected in 2012	12 April 2013



APPENDIX C: COMMUNITY CONSULTATION

C1. MEDIA RELEASE

Image C 1: Media Release, 12th September 2012



Wednesday, 12 September 2012

Flood stories wanted for Narooma catchment study

Eurobodalla Shire Council together with WMA Water is seeking your stories and photos on flooding in the Wagonga Inlet and Mummuga and Kianga lakes for a study they are preparing on the catchment areas.

"We are after local knowledge and personal experiences from residents and businesses to contribute to the development of a flood study," said Eurobodalla Shire Council's Planning and Sustainability Director, Lindsay Usher.

The catchments around Narooma are known to be at risk from flooding and Mr Usher says the study will help council meet its responsibilities under the NSW Government's Flood Prone Land Policy and also help council to plan for and manage flood risks in these areas.

"Community involvement in the study is vital. We are specifically interested in any historical records of flooding like photographs and flood marks and in hearing people's stories about floods in these catchments," Mr Usher said.

A newsletter and questionnaire will soon be mailed to residents within each catchment although people do not need to live in the flood prone areas to participate, says Mr Usher. "Anyone with flood information on these areas can jump online and complete the questionnaire."

The questionnaire can be completed at https://www.surveymonkey.com/s/W5VZV9R or via a link on council's homepage, www.esc.nsw.gov.au. Hard copies of the survey are available at Narooma library.

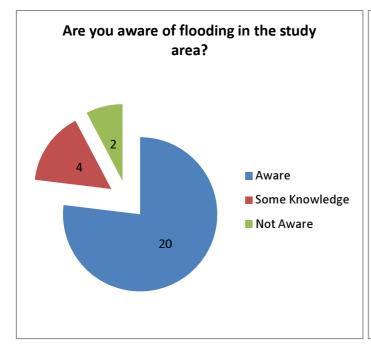
Residents can also bring their photos and stories of past flood events to the WMA Water information booth at Narooma library on Monday 17 September, between 4.00 and 7.00pm.

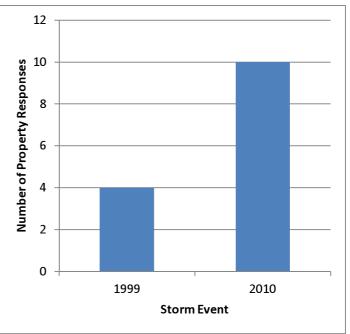
Residents wanting more information on the flood study are invited to contact Eurobodalla Shire Council's coastal and flood planner, Norm Lenehan, on 4474 1374 or via email to norman.lenehan@eurocoast.nsw.qov.au.

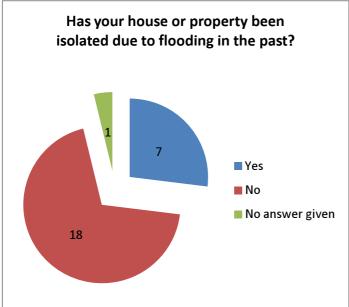
For all **media enquiries**, please contact Eurobodalla Shire Council, on T 4474 1012, M 0408 200 191 or via email to council.media@eurocoast.nsw.gov.au



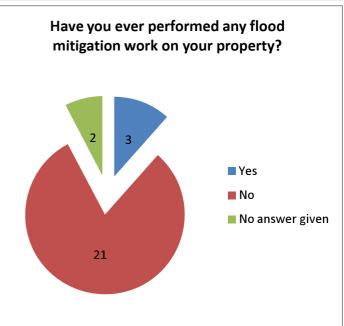
FIGURE C1 COMMUNITY RESPONSE ANAYLSIS

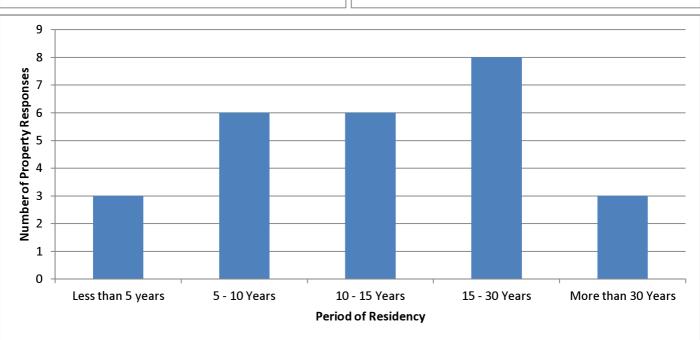






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Item No.	Respondent	Catchment	Issue	Response	Changes to Report
				The Flood Study aims to define flood behaviour and identify parcels of land	
				which are subject to inundation during the 1% AEP event. Properties are	
				classified as flood affected if any part of the total land area within the property	
				boundary is affected by inundation from the 1% AEP (or 100 year ARI) event.	
				This is irrespective of the location of the dwellings or buildings on the	The terminology within the Flood Study report relating
		Wagonga	Identification of "property" as flooded, buildings are	property. These lots or parcels of land have been referred to as properties in	to parcel of land, lot, property or building has been
1	Joan Lynch	Inlet	20m above lake	the flood study report.	clarified.
			Clearly define what impact the flood study has on their	The Flood Study aims to define flood behaviour, this information will allow	
			land, advised to build back so not in flood line. Has this	Council to provide landowners with accurate information regarding their flood	
2	Peter and Eva Wilson	Duck Pond	changed?	affectation.	Nil
			Flood level information and descriptions provided for	Additional information regarding historical flood events has come to light, this	Addition of the 2014 event to the historical events
		Mummaga	January 1999, February 2010 and October 2014 events.	has allowed the flood model to be calibrated and provide further confidence in	investigated and further documentation of the
3.1	J Keenan	Lake		the model results.	calibration process in Section 8
				The Flood Study aims to define flood behaviour. The second stage in the	
				Floodplain Risk Management Process is the Floodplain Risk Management Study	
				(that is yet to be undertaken). The purpose of the Floodplain Risk	
				Management Study is to evaluate flood management and mitigation options	
				for the catchment. During this stage it is possible to investigate the benefit of	
2.2	LIZ	Mummaga	Character I Flood with a time to a consultant and	options such as enclosing the stormwater channel adjacent to your property,	N.:I
3.2	J Keenan	Lake Mummaga	Channel Flood mitigation works suggested.	reviewing berm opening policies or modifications to structures.	Nil
11	R Peady	Lake	Title of Figure E17C is incorrect.	Figure title has been amended.	Title of Figure E17C has been amended.
4.1	it ready	Mummaga	Flood level information for the February 2010 event	I igure title has been amended.	Title of Figure L17C has been amended.
4.2	R Peady	Lake	provided.	Refer to Response 3.1	Refer to Response 3.1
7.2	Kircuay	Mummaga	Review of berm opening triggers required to minimise	Neier to Nesponse 3.1	Neter to response 5.1
4.3	R Peady	Lake	flood damage.	Refer to Response 3.2	Nil
		Mummaga	Review of berm opening triggers required to minimise		
5	I Peady	Lake	flood damage.	Refer to Response 3.2	Nil
	,		Flood level information and descriptions provided for		
6.1	P Hattersley	Duck Pond	February 2010 event.	Refer to Response 3.1	Refer to Response 3.1
			Identification of "property" as flooded. A number of		
6.2	P Hattersley	Duck Pond	properties are only impacted to a minimal extent.	Refer to Response 1	Refer to Response 1
				Flood emergency response classifications for communities are developed in	
			Classification as Rising Road Access perceived as	accordance with NSW SES guidelines. Classifications aim to assist the NSW SES	
6.3	P Hattersley	Duck Pond	alarming	in targeting emergency functions during events up to and including the PMF.	Section 13.4 reworded to provide clarification.
			Terminology related to description of Dalmeny Drive		
6.4	P Hattersley	Duck Pond	structure.	-	Clarification is provided in Section 1.1
_			Potential impacts of the Dalmeny Drive structure on		l
6.5	P Hattersley	Duck Pond	flooding should be investigated.	Refer to Response 3.2	Nil
6.6	P Hattersley	Duck Pond	Duck Ponds classification as an ICOLL.	Mummuga and Kianga Lakes are classified as ICOLLs by the NSW Government.	Clarification is provided in Section 1.1 and 7.1

			T		
			Raising of the concrete structure upstream of Dalmeny		
6.7	P Hattersley	Duck Pond	Drive to maintain significant ecological habitat.	Refer to Response 3.2	Nil
	•		ů ů		
				The acquisition of large scale topography data is a large project and as such it	
				is common industry practice to use the best available information and	
				supplement this with field survey of more recent changes that may impact the	
			Use of terrain data from 2012 suggests project is out of	flood behaviour. It is common that such a review is undertaken as part of the	
6.8	P Hattersley	Duck Pond	date.	Floodplain Risk Management Study.	Nil
			Statement of aim of Flood Study to define the current	The primary aim of the flood study is to define flood behaviour under current	
6.9	P Hattersley	Duck Pond	flood behaviour	catchment conditions at the studies commencement.	Clarification is provided in Section 1.2
				The Flood Study aims to define flood behaviour. It is a technical investigation	
				of flood behaviour in the various catchments, the outcomes of the flood study	
7 1	Ian Hitchcock	Mummaga Lake		are presentation of the flood behaviour. Typically a flood study does not	An avagutiva summanu has baan addad
7.1	Idii Hitchcock	Lake	Discrepancy between modelled levels and actual peak	contain recommendations for the management of flooding or conclusions.	An executive summary has been added.
		Mummaga	flood levels for historical events. Information related to		
7.2	Ian Hitchcock	Lake	historical events provided.	Refer to Response 3.1	Refer to Response 3.1
7.2	Tall Titlechedek	Luike	installed events provided.	There to response 5.2	Neter to response 3.1
				The rainfall patterns for the eastern catchment areas of Mummaga Lake were	
				calibrated by recordings at pluviometers located at the Narooma (1999 event)	
				and Barlows Bay (2007, 2010 events). Rainfall patterns for the western sub-	
				catchments of Mummaga Lake were calibrated with the Tuross recordings.	
				The recorded rainfall patterns from the Captains Flat radar were used to verify	
				pluviometer patterns (at Narooma, Barlows Bay and Tuross) and was not	
				directly applied to the models. The recorded rainfall patterns were found to	
				have replicated well across the catchments. The radar is useful in confirming	
		Mummaga	Use of rainfall information from Tuross and Captains	rainfall duration and intensity and is used as an additional analysis tool to	
7.3	Ian Hitchcock	Lake	Flat	support the recorded rainfall data.	Nil
				No survey data were available to determine the precise height of the	
				Mummaga Lake sand berm at the commencement of the 2010 flood event. It	
				is known the entrance was closed at the commencement of the event and a	
				natural break out occurred. Given no survey data were available, it was	
				assumed the entrance opened at the management trigger level of 1.175mAHD.	
				No survey data was available to accurately determine the lake level at the commencement of the event therefore an assumption needed to be made.	
				The model indicates peak levels in Mummaga Lake are relatively insensitive to	
		Mummaga	Assumptions used regarding lake starting level and	variations in the Initial Water Level as demonstrated in Table 26 on page 61 of	
7.4	Ian Hitchcock	Lake		the draft study.	Nil
		1	· · · · · · · · · · · · · · · · · · ·	1	I ^{****}

				The flood study modelled a range of entrance conditions. Please refer to page	
				50 and page 60 of the report that describes the entrance conditions modelled.	
				The National Parks and Wildlife Service (NPWS) has management responsibility	
				for the entrance of Mummaga Lake as part of the Eurobodalla National Park.	
		Mummaga	Failure for the berm to be artificially opened in	This flood study may provide additional information to assist with managing	
7.5	Ian Hitchcock	Lake	historical events.	the entrance.	Nil
		Mummaga		The flood study assessed the sensitivity of design flood levels in combination	
7.6	Ian Hitchcock	Lake	The sea level issues has not been properly addressed.	with climate change scenarios. Please refer to Section 14.	Nil
		Mummaga		The study has been undertaken in accordance with standard industry practice,	
7.7	Ian Hitchcock	Lake	Independent Peer Review requested	using established methods.	Nil
				Australian Rainfall and Runoff provides national guidance on flood estimation.	
				The 1987 version and the current recently revised version of Australian Rainfall	
				and Runoff and the National Flood Manual (Managing the floodplain: a guide	
				to best practice in flood risk management in Australia), encourage the	
		Mummaga		dissemination of the best available information on flood behaviour so that	
7.0	lan Hitchand	_	Use of Stage 1 results to set EDI		Nii
7.8	Ian Hitchcock	Lake	Use of Stage 1 results to set FPL	flood risk can be understood and actively managed.	Nil
7.0	Lea IPI de cal	Mummaga	T'll (5') 5470 '- '	Defects Decree 44	Defects Berry A4
7.9	Ian Hitchcock	Lake	Title of Figure E17C is incorrect.	Refer to Response 4.1	Refer to Response 4.1
		Mummaga		Local consultation has occurred as follows:	
7.10	Ian Hitchcock	Lake	Adequate community consultation was not undertaken		Nil
				Media release "Flood Stories wanted for Narooma catchment study"	
				published 12 September 2012	
				Questionnaire and information sheet distributed to local residents and	
				businesses September 2012	
				On-line "survey monkey" publicised in information sheet	
				Community drop-in session 17 September 2012	
				Council report seeking endorsement to exhibit draft plan 9 June 2015	
				Website notice of exhibition and public meeting 12 June 2015	
				Media release 10 June 2015 Narooma News	
				Mail out to all residents within identified flood planning area 15 June 2015	
				Eurobodalla News – advice on public information sessions and exhibition	
				3 July	
				Website notice "what's on" of public meeting	
				·	
				Follow-up media release 8 July Narooma News (website 8 July) Public information page 20 July 2015	
				Public information session 8 July 2015 Public information session 8 July 2015	
				Business information session 8 July 2015	
		N 4	27 Manage Charact agreed level at 4.0 ag AUD and 3th ag	Public information session 9 July 2015. The land of this session 9 July 2015. The land of this session 9 July 2015. The land of this session 9 July 2015.	
7.44	Lea IPI de cal	Mummaga	37 Myuna Street garage level at 1.8 mAHD and it has	The level of this garage was surveyed at 2.227 mAHD. This level was used in	API
/.11	Ian Hitchcock	Lake	never been inundated.	the calibration process.	Nil
	La a 119 ala a a 1	Mummaga	Further information provided in relation to historic	D. C. J. D	Bufulla Burus 24
7.12	Ian Hitchcock	Lake	events including 2014.	Refer to Response 3.1	Refer to Response 3.1
_		Mummaga	Why was the 2014 event not included in the Flood		
7.13	Ian Hitchcock	Lake	Study	Refer to Response 3.1	Refer to Response 3.1

		Mummaga	Berm management and some drainage structure		
7.14	an Hitchcock	Lake	inadequate.	Refer to Response 3.2	Nil



APPENDIX D: MATRIX OF SCENARIOS

D1. WAGONGA INLET

	Rainfall Probability	Ocean Probability	Ocean Conditions – Initial Water Level	Ocean Conditions – Continuing	Entrance Conditions – Initial	Entrance Conditions – Continuing	Lake Conditions – Initial Water Level
Historic Flood Modelling							
2008 (Calibration Tidal)	2008 Rainfall (No Rainfall)	2008 Tide	+ 0.346 m AHD	As recorded	Training Wall– 90% Impervious	Constant	+0.07 m AHD (Narooma Wharf) -0.18 m AHD (Barlows Bay)
1999 (Calibration Rainfall)	1999 Rainfall	1999 Tide	+ 0.454 m AHD	As recorded	Training Wall– 90% Impervious	Constant	+0.23 m AHD (Narooma Wharf) +0.03 m AHD (Barlows Bay)
2007 (Validation Rainfall)	2007 Rainfall	2007 Tide	+ 0.107 m AHD	As recorded	Training Wall– 90% Impervious	Constant	+0.04 m AHD (Narooma Wharf) +0.06 m AHD (Barlows Bay)
2010 (Validation Rainfall)	2010 Rainfall	2010 Tide	- 0.565 m AHD	As recorded	Training Wall– 90% Impervious	Constant	-0.08 m AHD (Narooma Wharf) -0.24 m AHD (Barlows Bay)
Historic Flood Modelling – Sens	sitivity						
Tide Level (without 0.1 m decrease)	No Rainfall	2008 Tide (without 0.1 m decrease)	+ 0.446 m AHD	As recorded	Training Wall– 90% Impervious	Constant	+0.07 m AHD (Narooma Wharf) -0.18 m AHD (Barlows Bay)
Training Wall Gaps – 100% Impervious	No Rainfall	2008 Tide	+ 0.346 m AHD	As recorded	Training Wall– 100% Impervious	Constant	+0.07 m AHD (Narooma Wharf) -0.18 m AHD (Barlows Bay)
Training Wall Gaps – 50% Impervious	No Rainfall	2008 Tide	+ 0.346 m AHD	As recorded	Training Wall– 50% Impervious	Constant	+0.07 m AHD (Narooma Wharf) -0.18 m AHD (Barlows Bay)
Design Flood Modelling – Critic	al Storm Duration						
1% AEP Rainfall Event	1% AEP Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Training Wall– 90% Impervious	Constant	0.60 m AHD
PMF Rainfall Event	PMF Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Training Wall– 90% Impervious	Constant	0.60 m AHD
Design Flood Modelling							
HHWS Ocean Event	No Rainfall	HHWS	0.445 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
5% AEP Ocean Event	No Rainfall	5% AEP Elevated Ocean Level	0.64 m AHD	1.90 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
1% AEP Ocean Event	No Rainfall	1% AEP Elevated Ocean Level	0.66 m AHD	2.00 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
20% AEP Rainfall Event	20% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
10% AEP Rainfall Event	10% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
5% AEP Rainfall Event	5% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
2% AEP Rainfall Event	2% AEP Rainfall	5% AEP Elevated Ocean Level	0.95 m AHD	1.90 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
	1% AEP Rainfall	5% AEP Elevated Ocean Level	0.95 m AHD	1.90 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
1% AEP Rainfall Event	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.01 m AHD	2.00 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
	1% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level
0.5% AEP Rainfall Event	0.5% AEP Rainfall	1% AEP Elevated Ocean Level	1.01 m AHD	2.00 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level

PMF Rainfall Event	PMF Rainfall	1% AEP Elevated Ocean Level	1.01 m AHD	2.00 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
Design Flood Modelling - Sens	Design Flood Modelling - Sensitivity										
Roughness Variation	1% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
Climate Change (Rainfall Increase)	1% AEP Rainfall Increased	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
Climate Change (Sea Level Rise)	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.05 m AHD 1.18 m AHD 1.34 m AHD 1.67 m AHD	2.00 m AHD (2030 Scenario) 2.13 m AHD (2050 Scenario) 2.29 m AHD (2070 Scenario) 2.62 m AHD (2100 Scenario)	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.11 m AHD 1.24 m AHD 1.40 m AHD 1.73 m AHD	2.10 m AHD (2030 Scenario) 2.23 m AHD (2050 Scenario) 2.39 m AHD (2070 Scenario) 2.72 m AHD (2100 Scenario)	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
Timing of Ocean Peak	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.46 m AHD (- 3 hrs) 0.07 m AHD (+ 3 hrs)	1.90 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				
	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.53 m AHD (- 3 hrs) 0.09 m AHD (+ 3 hrs)	2.00 m AHD Peak	Training Wall– 90% Impervious	Constant	Ocean – Initial Water Level				

D2. KIANGA LAKE

	Rainfall Probability	Ocean Probability	Ocean Conditions –	Ocean Conditions -	Entrance Conditions –	Entrance Conditions –	Lake Conditions –		
Historia Flood Modelling			Initial Water Level	Continuing	Initial	Continuing	Initial Water Level		
Historic Flood Modelling									
1999 (Calibration Rainfall)	1999 Rainfall	1999 Tide	+ 0.454 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Breakout: Commences at 2.0 m AHD Duration of 4 hours	0.6 m AHD		
2007 (Validation Rainfall)	2007 Rainfall	2007 Tide	+ 0.107 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Breakout: Commences at 2.0 m AHD Duration of 4 hours	0.6 m AHD		
2010 (Validation Rainfall)	2010 Rainfall	2010 Tide	- 0.565 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Breakout: Commences at 2.0 m AHD Duration of 4 hours	0.6 m AHD		
Historic Flood Modelling - Sens	itivity								
IWL 2.0 m AHD	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Constant (i.e. Closed)	2.0 m AHD		
IWL 1.0 m AHD	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Constant (i.e. Closed)	1.0 m AHD		
Entrance Open	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Open Entrance	Constant (i.e. Open)	Ocean – Initial Water Level		
Entrance Closed	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance (2.0 m AHD)	Constant (i.e. Closed)	2.0 m AHD		
Breakout Duration 2 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 2.0 m AHD Duration of 2 hours	0.6 m AHD		
Breakout Duration 6 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 2.0 m AHD Duration of 6 hours	0.6 m AHD		
Breakout Duration 12 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 2.0 m AHD Duration of 12 hours	0.6 m AHD		
Design Flood Modelling - Critic	al Storm Duration								
1% AEP Rainfall Event	1% AEP Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD		

PMF Rainfall Event	PMF Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD
Design Flood Modelling							
HHWS Ocean Event	No Rainfall	HHWS	0.445 m AHD	0.995 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level
5% AEP Ocean Event	No Rainfall	5% AEP Elevated Ocean Level	0.640 m AHD	2.35 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level
1% AEP Ocean Event	No Rainfall	1% AEP Elevated Ocean Level	0.66 m AHD	2.55 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level
20% AEP Rainfall Event	20% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
10% AEP Rainfall Event	10% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
5% AEP Rainfall Event	5% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
2% AEP Rainfall Event	2% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
1% AEP Rainfall Event	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
	1% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
0.5% AEP Rainfall Event	0.5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
PMF Rainfall Event	PMF Rainfall	1% AEP Elevated Ocean Level	2.45 m AHD (45min, 60min) 2.16 m AHD (2 hr)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
Design Flood Modelling - Sensi	itivity						
Roughness Variation	1% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Closed Entrance (2.0 m AHD)	Constant	0.6 m AHD
Climate Change (Rainfall Increase)	1% AEP Rainfall Increased	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Closed Entrance (2.0 m AHD)	Constant	0.6 m AHD
Climate Change (Sea Level Rise)	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.12 m AHD 1.25 m AHD 1.41 m AHD 1.74 m AHD	2.45 m AHD (2030 Scenario) 2.58 m AHD (2050 Scenario) 2.74 m AHD (2070 Scenario) 3.07 m AHD (2100 Scenario)	Closed Entrance (2.0 m AHD)	Constant	0.6 m AHD
Timing of Ocean Peak	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD (- 3 hrs) 1.97 m AHD (+ 3 hrs)	2.35 m AHD Peak	Closed Entrance (2.0 m AHD)	Constant	0.6 m AHD
Timing of Ocean Peak	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD (- 3 hrs) 2.16 m AHD (+ 3 hrs)	2.55 m AHD Peak	Closed Entrance (2.0 m AHD)	Constant	0.6 m AHD

D3. MUMMUGA LAKE

	Rainfall Probability	Ocean Probability	Ocean Conditions – Initial Water Level	Ocean Conditions – Continuing	Entrance Conditions – Initial	Entrance Conditions – Continuing	Lake Conditions – Initial Water Level
Historic Flood Modelling							
1999 (Calibration Rainfall)	1999 Rainfall	1999 Tide	+ 0.454 m AHD	As recorded	Closed Entrance (1.175 m AHD)	Breakout: Commences at 1.175 m AHD Duration of 4 hours	0.6 m AHD
2007 (Validation Rainfall)	2007 Rainfall	2007 Tide	+ 0.107 m AHD	As recorded	Closed Entrance (1.175 m AHD)	Breakout: Commences at 1.175 m AHD Duration of 4 hours	0.6 m AHD
2010 (Validation Rainfall)	2010 Rainfall	2010 Tide	- 0.565 m AHD	As recorded	Closed Entrance (1.175 m AHD)	Breakout: Commences at 1.175 m AHD Duration of 4 hours	0.6 m AHD
Historic Flood Modelling - Sen	sitivity						
IWL 1.175 m AHD	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Constant (i.e. Closed)	1.175 m AHD
Entrance Open	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Open Entrance	Constant (i.e. Open)	Ocean – Initial Water Level
Entrance Closed	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Constant (i.e. Closed)	1.175 m AHD

Breakout Duration 2 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 1.175 m AHD Duration of 2 hours	0.6 m AHD		
Breakout Duration 4 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 1.175 m AHD Duration of 4 hours	0.6 m AHD		
Breakout Duration 6 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 1.175 m AHD Duration of 6 hours	0.6 m AHD		
Breakout Duration 12 hrs	2010 Rainfall	2010 Tide	-0.565 m AHD	As recorded	Closed Entrance	Breakout: Commences at 1.175 m AHD Duration of 12 hours	0.6 m AHD		
Design Flood Modelling - Criti	cal Storm Duration								
1% AEP Rainfall Event	1% AEP Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD		
PMF Rainfall Event	PMF Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD		
Design Flood Modelling									
HHWS Ocean Event	No Rainfall	HHWS	0.445 m AHD	0.995 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
5% AEP Ocean Event	No Rainfall	5% AEP Elevated Ocean Level	0.64 m AHD	2.35 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
1% AEP Ocean Event	No Rainfall	1% AEP Elevated Ocean Level	0.66 m AHD	2.55 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
20% AEP Rainfall Event	20% AEP Rainfall	HHWS + Neap Tide	0.845 m AHD (120min) -0.295 m AHD (540min) 0.075 m AHD (2880min)	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
10% AEP Rainfall Event	10% AEP Rainfall	HHWS + Neap Tide	0.845 m AHD (120min) -0.295 m AHD (540min) 0.075 m AHD (2880min)	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
2% AEP Rainfall Event	2% AEP Rainfall	5% AEP Elevated Ocean Level	1.97 m AHD (120min) 1.31 m AHD (540min) 1.23 m AHD (2880min)	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.97 m AHD (120min) 1.31 m AHD (540min) 1.23 m AHD (2880min)	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
1% AEP Rainfall Event	5% AEP Rainfall	1% AEP Elevated Ocean Level	2.16 m AHD (120min) 1.47 m AHD (540min) 1.35 m AHD (2880min)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
	1% AEP Rainfall	HHWS + Neap Tide	0.845 m AHD (120min) -0.295 m AHD (540min) 0.075 m AHD (2880min)	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
0.5% AEP Rainfall Event	0.5% AEP Rainfall	1% AEP Elevated Ocean Level	2.16 m AHD (120min) 1.47 m AHD (540min) 1.35 m AHD (2880min)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
PMF Rainfall Event	PMF Rainfall	1% AEP Elevated Ocean Level	2.16 m AHD (120min) 1.47 m AHD (540min) 1.35 m AHD (2880min)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
Design Flood Modelling - Sens	Design Flood Modelling - Sensitivity								
Roughness Variation	1% AEP Rainfall	HHWS + Neap Tide	0.845 m AHD (120min) -0.295 m AHD (540min) 0.075 m AHD (2880min)	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		
Climate Change (Rainfall Increase)	1% AEP Rainfall Increased	HHWS + Neap Tide	0.845 m AHD (120min) -0.295 m AHD (540min) 0.075 m AHD (2880min)	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		

Climate Change (Sea Level Rise)	1% AEP Rainfall	5% AEP Elevated Ocean Level	2.07 m AHD (120min) 1.41 m AHD (540min) 1.33 m AHD (2880min)	2.45 m AHD (2030 Scenario)	- Closed Entrance		0.6 m AHD
			2.20 m AHD (120min) 1.54 m AHD (540min) 1.46 m AHD (2880min)	2.58 m AHD (2050 Scenario)		Constant	
			2.36 m AHD (120min) 1.70 m AHD (540min) 1.62 m AHD (2880min)	2.74 m AHD (2070 Scenario)			
			2.69 m AHD (120min) 2.03 m AHD (540min) 1.95 m AHD (2880min)	3.07 m AHD (2100 Scenario)			
Timing of Ocean Peak	1% AEP Rainfall	5% AEP Elevated Ocean Level	Add 3 hrs 2.26 m AHD (120min) 1.02 m AHD (540min) 0.40 m AHD (2880min)	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
			Minus 3 hrs 1.02 m AHD (120min) 1.78 m AHD (540min) 1.69 m AHD (2880min)				
	5% AEP Rainfall	1% AEP Elevated Ocean Level	Add 3 hrs 2.45 m AHD (120min) 1.17 m AHD (540min) 0.51 m AHD (2880min)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
			Minus 3 hrs 1.17 m AHD (120min) 1.96 m AHD (540min) 1.80 m AHD (2880min)	2.33 III AND Feak			

D4. DUCK POND

	Rainfall Probability	Ocean Probability	Ocean Conditions – Initial Water Level	Ocean Conditions – Continuing	Entrance Conditions – Initial	Entrance Conditions – Continuing	Lake Conditions – Initial Water Level		
Historic Flood Modelling									
1999 (Calibration Rainfall)	1999 Rainfall	1999 Tide	+ 0.454 m AHD	As recorded	Closed Entrance	Constant	0.6 m AHD		
2007 (Validation Rainfall)	2007 Rainfall	2007 Tide	+ 0.107 m AHD	As recorded	Closed Entrance	Constant	0.6 m AHD		
2010 (Validation Rainfall)	2010 Rainfall	2010 Tide	- 0.565 m AHD	As recorded	Closed Entrance	Constant	0.6 m AHD		
Design Flood Modelling - Critic	Design Flood Modelling - Critical Storm Duration								
1% AEP Rainfall Event	1% AEP Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD		
PMF Rainfall Event	PMF Rainfall	Constant Water Level	0.60 m AHD	0.60 m AHD	Closed Entrance	Constant	0.60 m AHD		
Design Flood Modelling									
HHWS Ocean Event	No Rainfall	HHWS	0.445 m AHD	0.995 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
5% AEP Ocean Event	No Rainfall	5% AEP Elevated Ocean Level	0.640 m AHD	2.35 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
1% AEP Ocean Event	No Rainfall	1% AEP Elevated Ocean Level	0.66 m AHD	2.55 m AHD Peak	Open Entrance	Constant	Ocean – Initial Water Level		
20% AEP Rainfall Event	20% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD		

10% AEP Rainfall Event	10% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
5% AEP Rainfall Event	5% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
2% AEP Rainfall Event	2% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
1% AEP Rainfall Event	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
	1% AEP Rainfall	HHWS + Neap Tide	-0.155 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
0.5% AEP Rainfall Event	0.5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
PMF Rainfall Event	PMF Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
Design Flood Modelling - Sens	itivity						
Roughness Variation	1% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
Climate Change	1% AEP Rainfall	HHWS + Neap Tide	0.075 m AHD	0.995 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
(Rainfall Increase)	Increased	HHWS + Neap Tide	0.075 III AI ID	0.995 III AI ID Feak	Closed Entrance	Constant	0.0 III ALID
Climate Change			1.12 m AHD 1.25 m AHD	2.45 m AHD (2030 Scenario)			
Climate Change (Sea Level Rise)	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.25 III AHD 1.41 m AHD	2.58 m AHD (2050 Scenario) 2.74 m AHD (2070 Scenario)	Closed Entrance	Constant	0.6 m AHD
(Sea Level Nise)			1.74 m AHD	3.07 m AHD (2100 Scenario)	Y		
Timing of Ocean Peak	1% AEP Rainfall	5% AEP Elevated Ocean Level	1.02 m AHD (- 3 hrs)	2.35 m AHD Peak	Closed Entrance	Constant	0.6 m AHD
			1.97 m AHD (+ 3 hrs)	2.00, 1.12			
	5% AEP Rainfall	1% AEP Elevated Ocean Level	1.17 m AHD (- 3 hrs) 2.16 m AHD (+ 3 hrs)	2.55 m AHD Peak	Closed Entrance	Constant	0.6 m AHD





Above: Flooding at the corner of Lynch Street and Brice Street at approximately 3pm



Above: Flooding of the Bowling Green and Bill Smyth Memorial Oval behind it at approximately 3pm



Above: Flooding on Bluewater Drive near the intersection with Bay Street



Above: Lynch Street, near the junction with Nichelsen Street (Narooma Newspaper)



Above: 8 Lynch Street, near the junction with Nichelsen Street



Above: McMillian Street



Above: Riverside Dr (Narooma Newspaper)

PHOTOGRAPHS OF FLOODING ON THE 15 FEBRAURAY 2010 DUCK POND AND KIANGA LAKE CATCHMENTS



Above: DUCK POND – Dalmeny Dr (Narooma Newspaper)



Above: DUCK POND – Eucalyptus Dr (Narooma Newspaper)



Above: DUCK POND – Junction of Eucalyptus Drive and Dalmeny Drive at approximately 3pm



Above: KIANGA LAKE – Junction of Princes Hwy and Kianga Rd (Narooma Newspaper)



Above: KIANGA LAKE – Junction of Dalmeny Dr and Centenary Dr (Narooma Newspaper



Above: KIANGA LAKE – Kianga Ck downstream of Princes Hwy (Narooma Newspaper



Above: KIANGA LAKE –Dalmeny Dr Bridge (Narooma Newspaper)



Above: Acacia Close

PHOTOGRAPHS OF FLOODING ON THE 15 FEBRAURAY 2010 **MUMMUGA LAKE CATCHMENT**



Above: Dalmeny Pedestrian Bridge, over Mummuga Lake on the16th February 2010



Above: Corner of Binalong Street, at approximately 10:30am



Above: Corner of Binalong Street



time unknown



Above: Mort Ave, Dalmeny Fire Brigade Hall (Narooma Newspaper)



Above: Mummuga Lake ICOLL entrance was open to the ocean at the time of photo



Above: Mort Ave, opposite the Dalmeny Fire Brigade Hall, at approximately 3pm

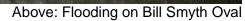
FIGURE E1D PHOTOGRAPHS OF FLOODING ON THE 14 OCTOBER 2014 WAGONGA INLET CATCHMENT















Above: Lynch Street, near the junction with Nichelsen Street





Above: Panorama at McMillian Road

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MUMMUGA LAKE CATCHMENT



Above: Flood water in the front yard of properties on Mort Avenue



Above: Channel between Mort Avenue and Tatiara Street (upstream of Mort Avenue)



Above: White-wash from flow discharging into Mummuga Lake from the channel between Mort Avenue and Tatiara Street (downstream of Mort Avenue)



Above: Flood water inside garage on Mort Avenue



Above: Channel between Mort Avenue and Tatiara Street (upstream of Mort Avenue)



Above: White-wash from flow discharging into Mummuga Lake from the channel between Mort Avenue and Tatiara Street (downstream of Mort Avenue)

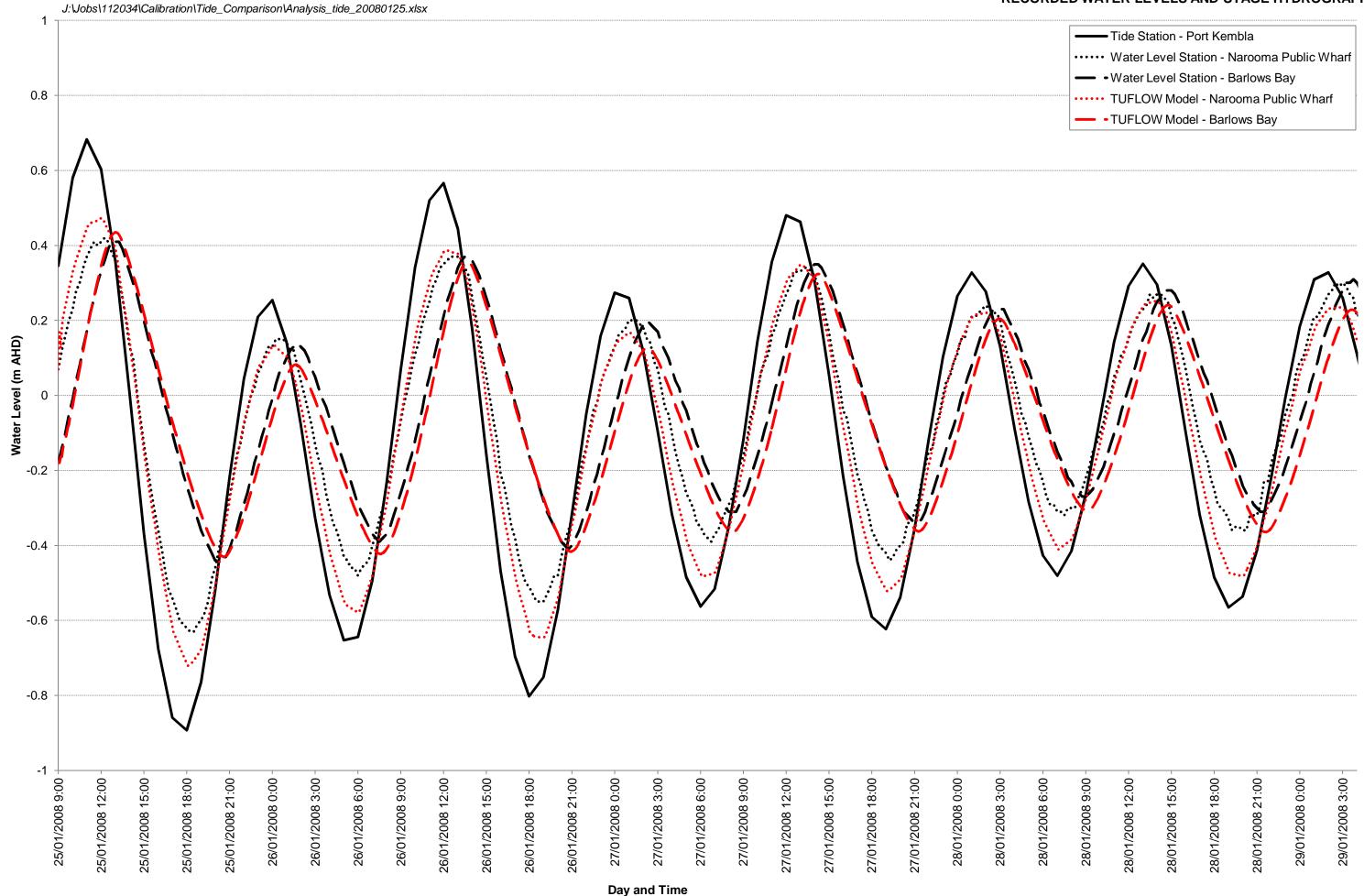
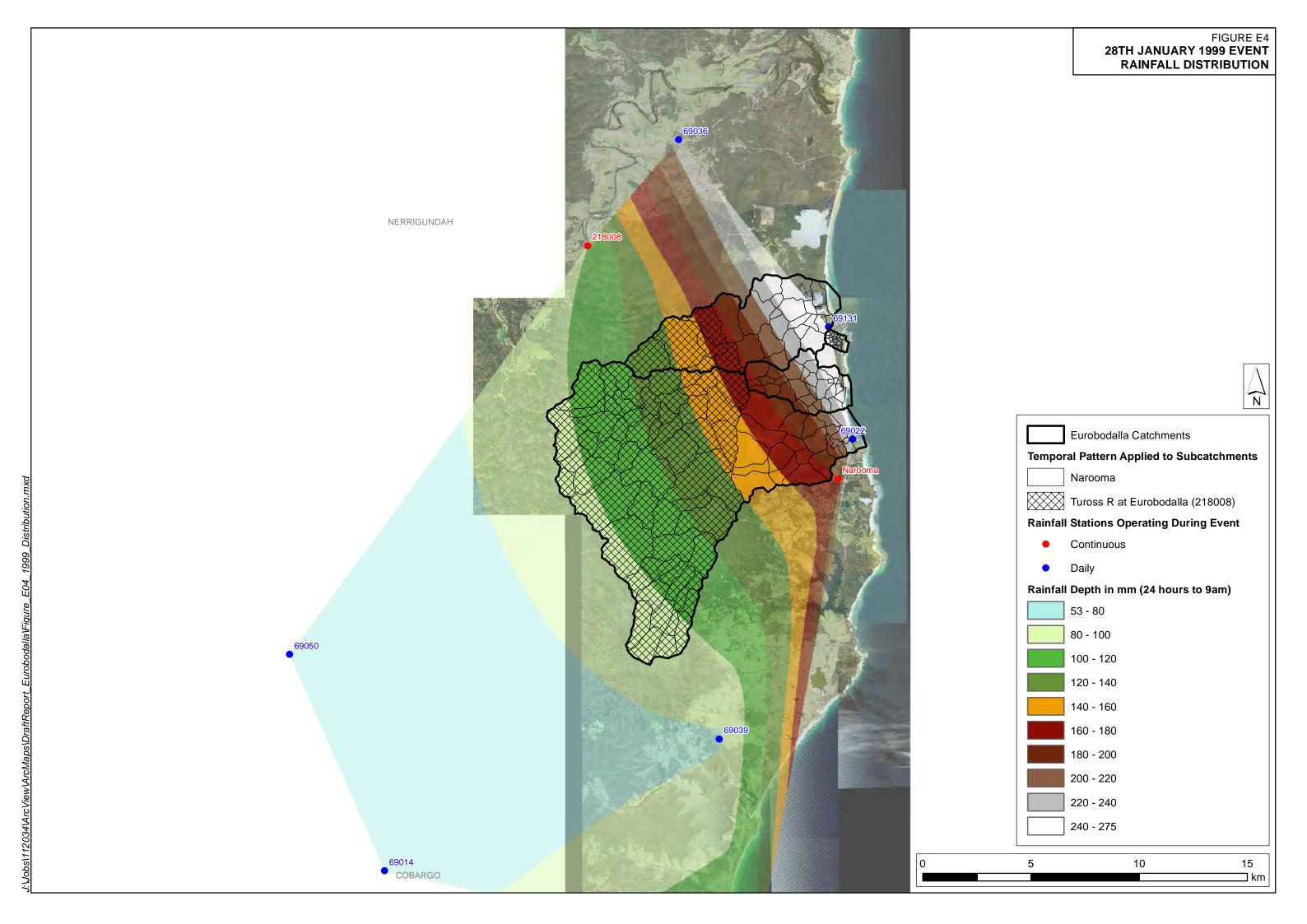
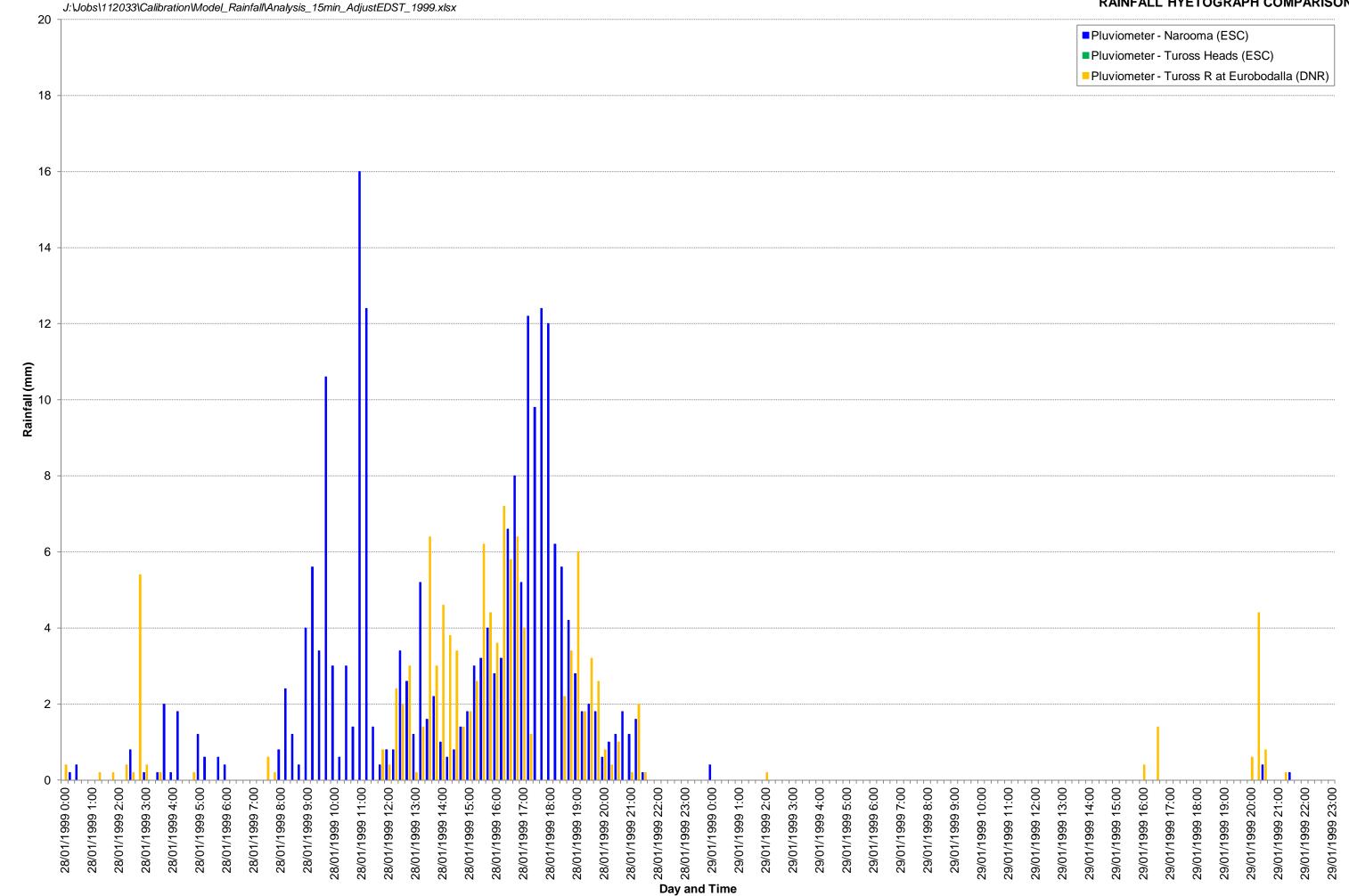
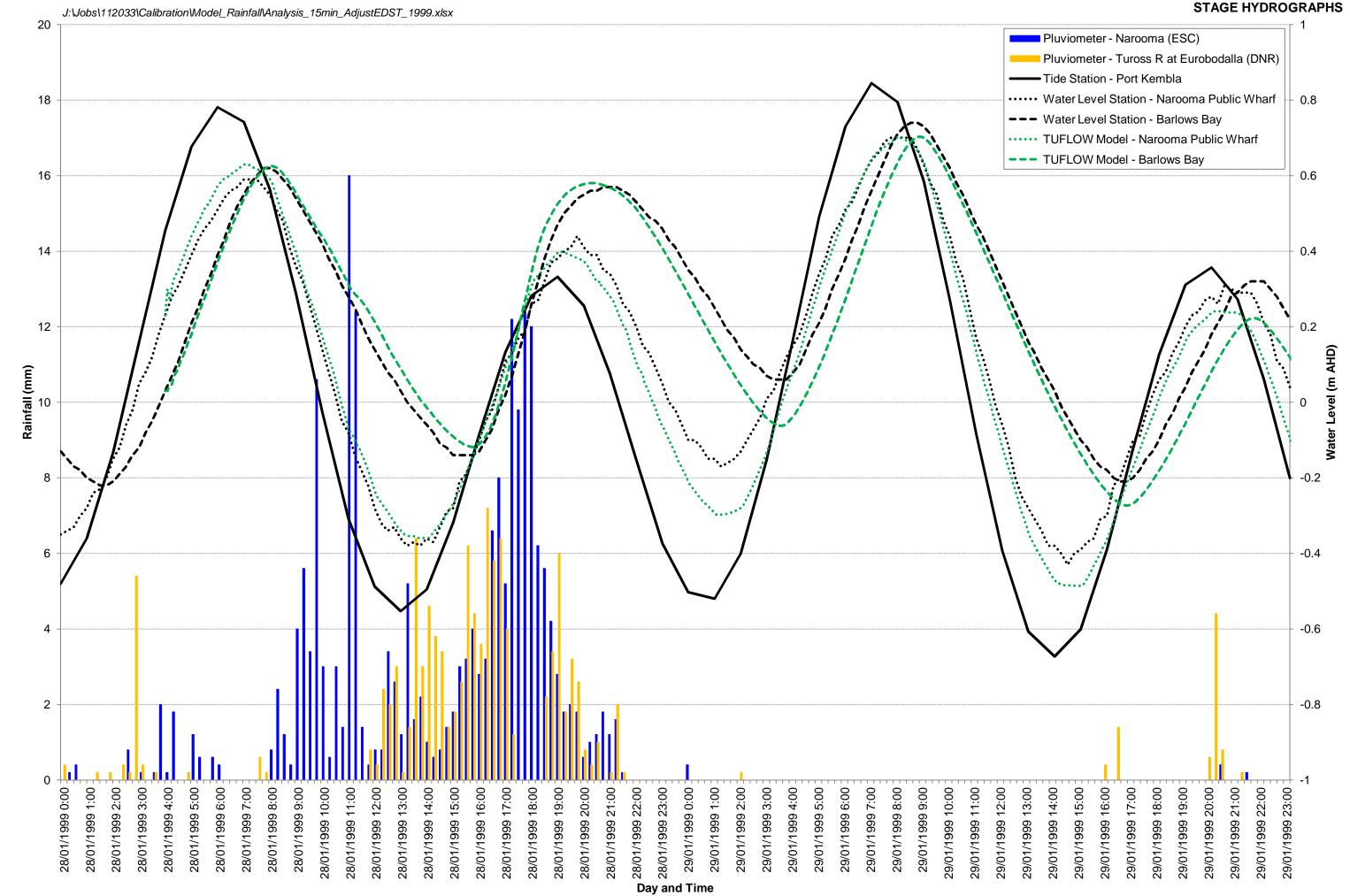
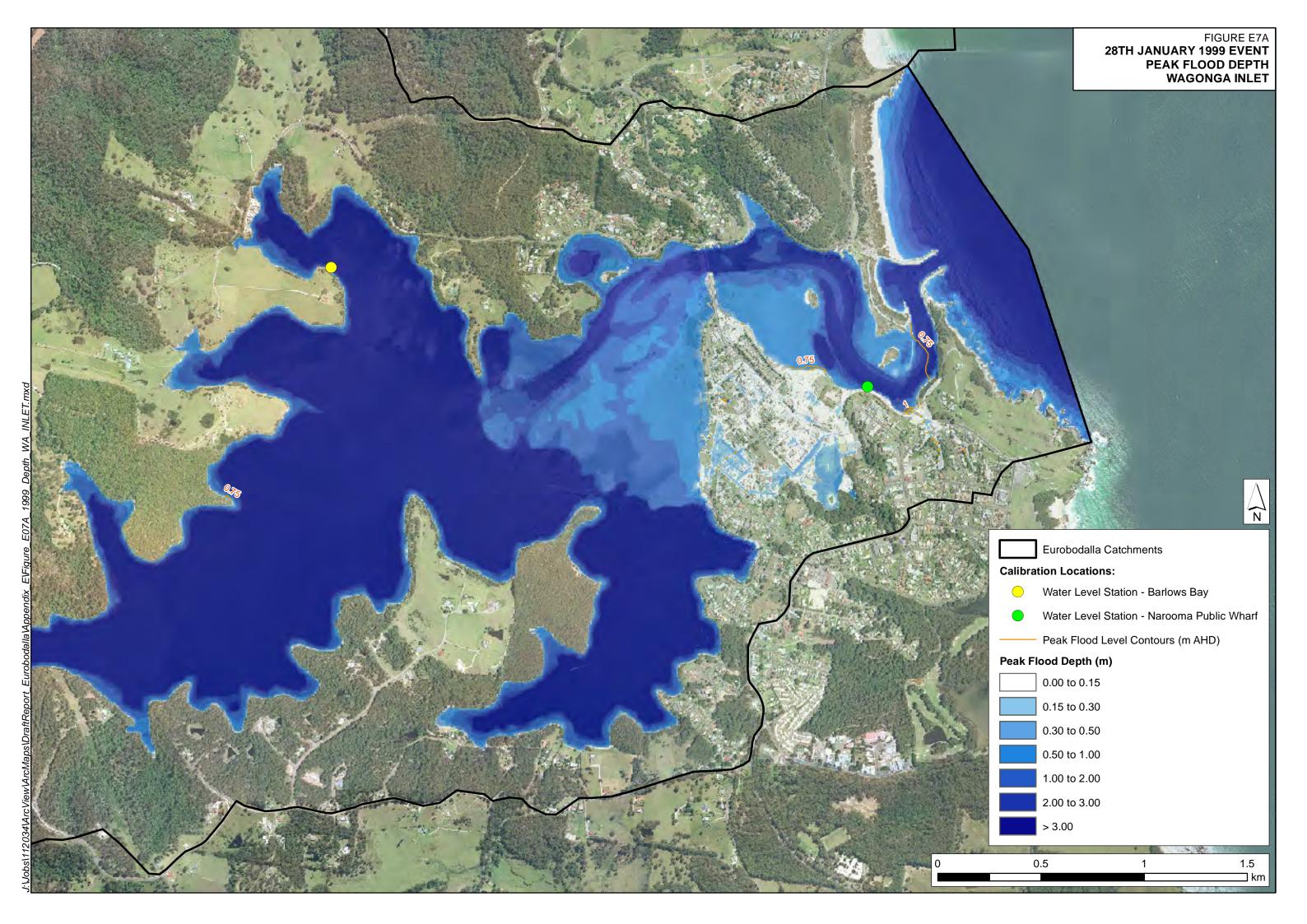


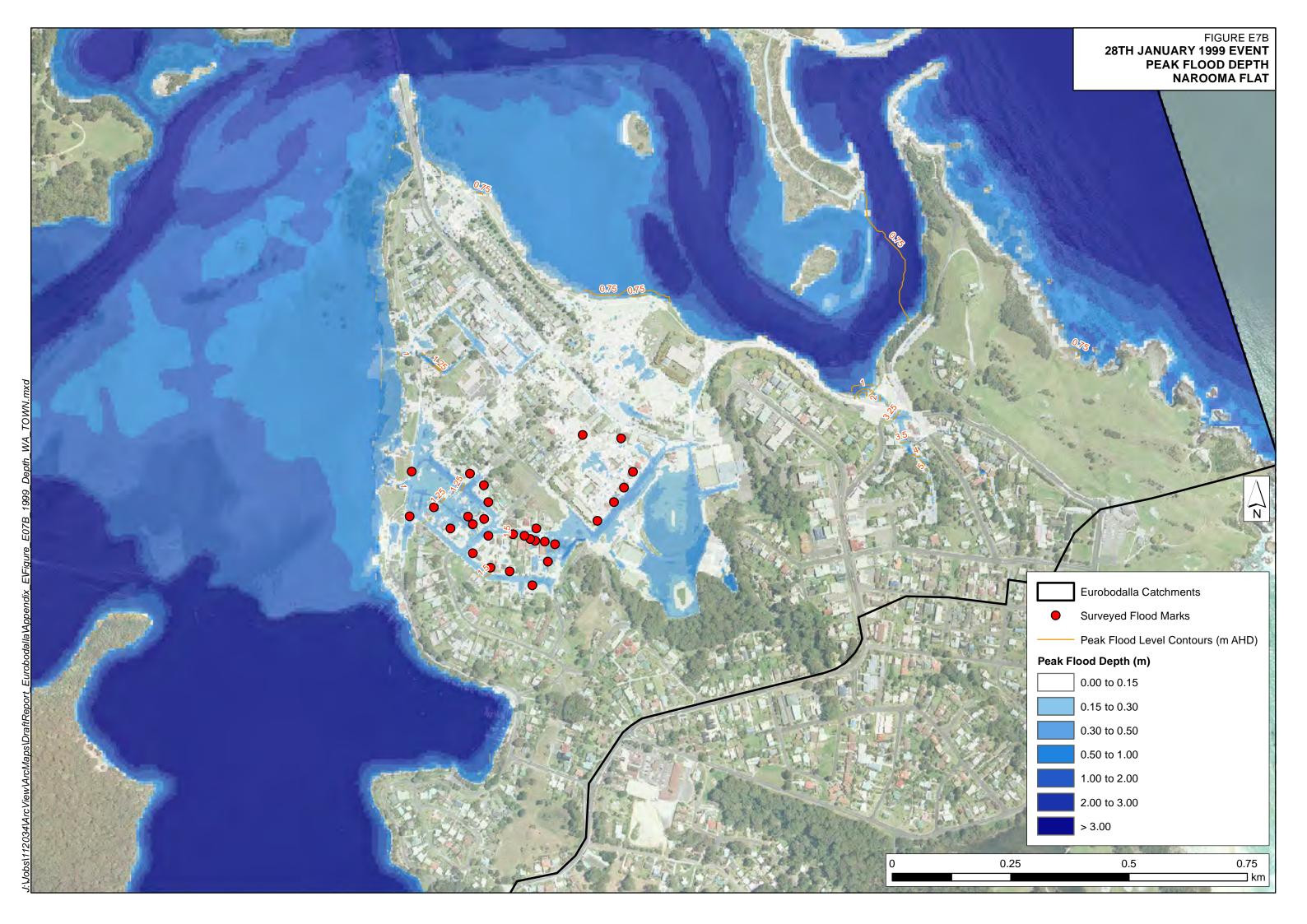
FIGURE E3
28TH JANUARY 1999 EVENT
IFD DATA

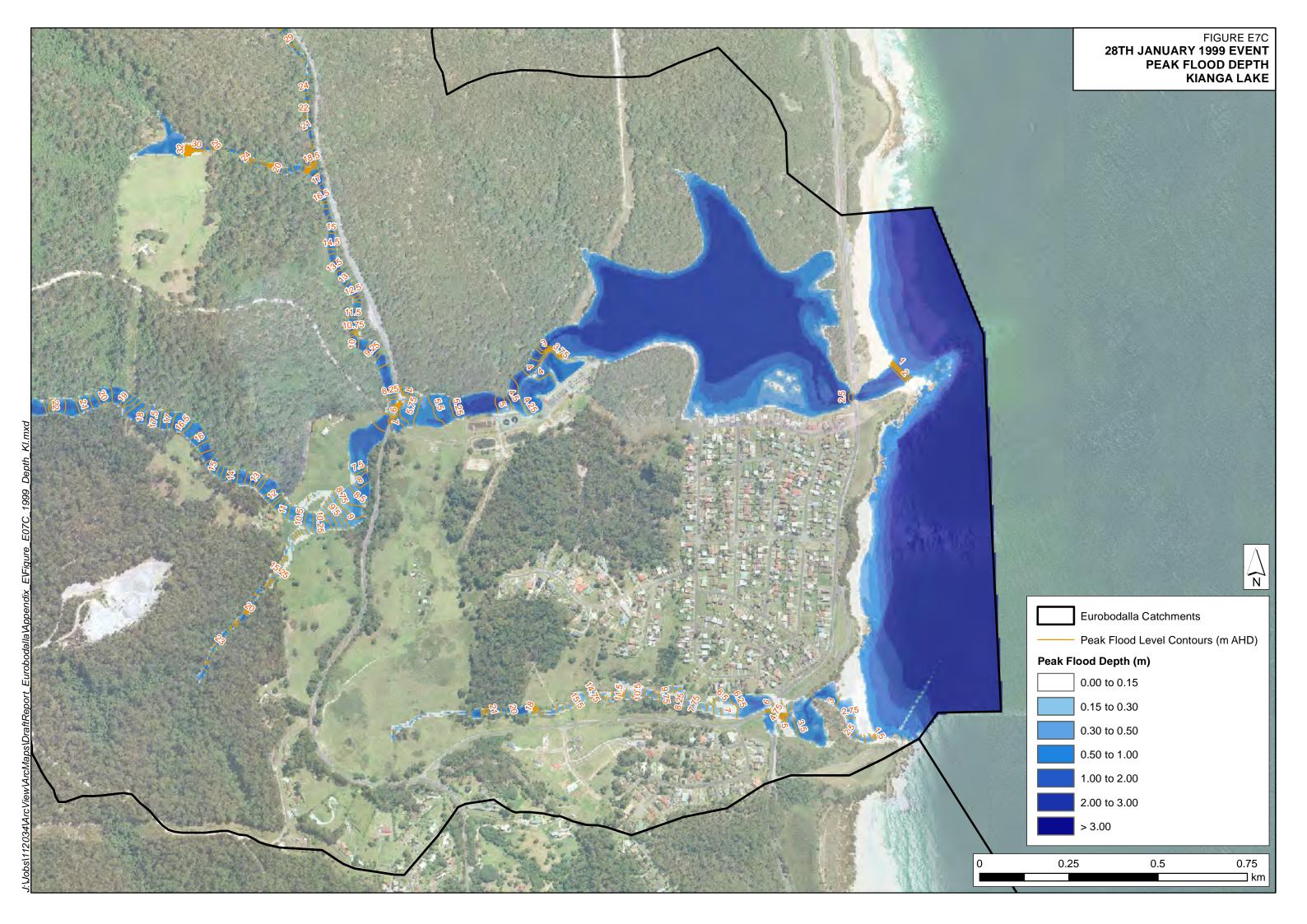


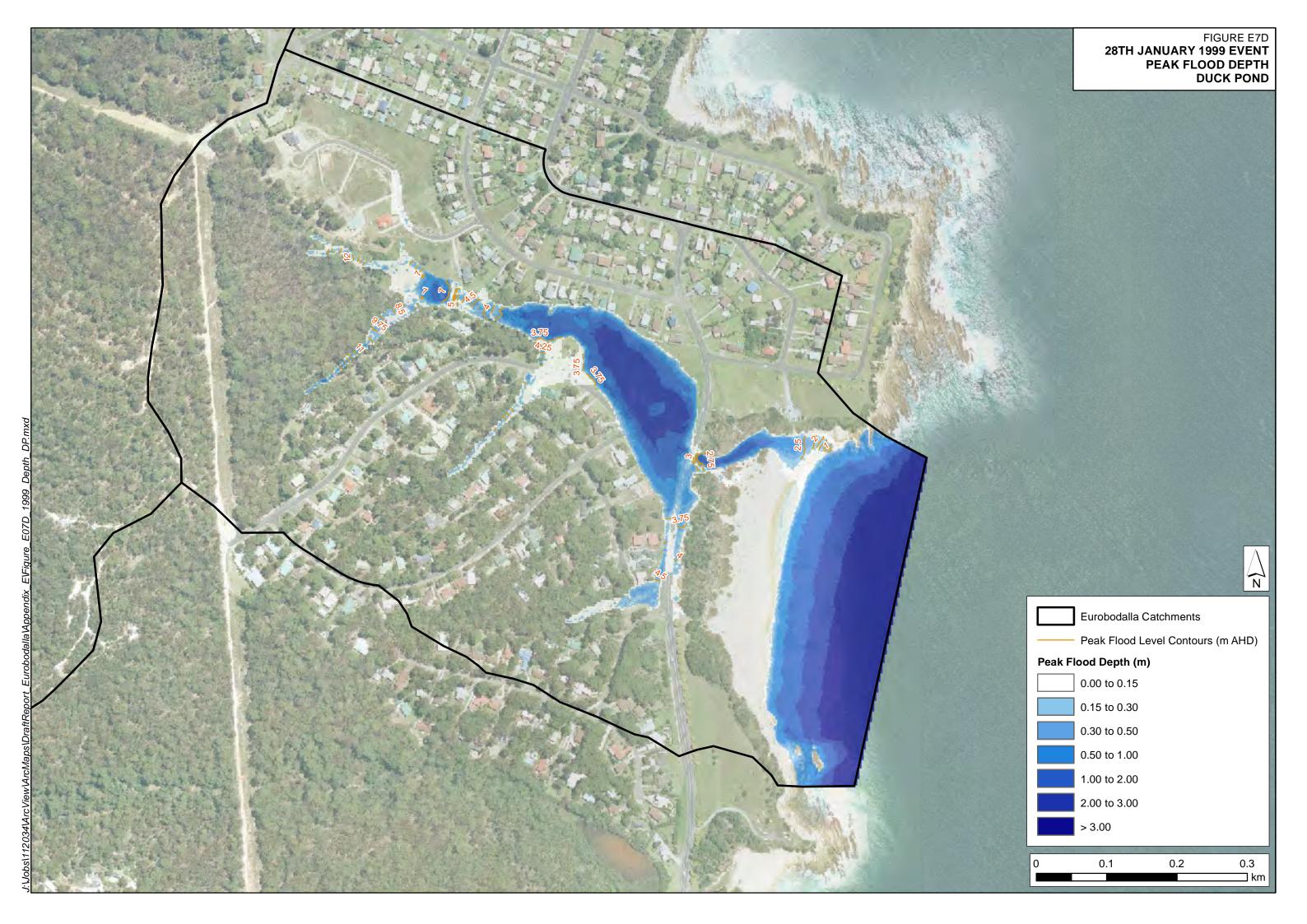


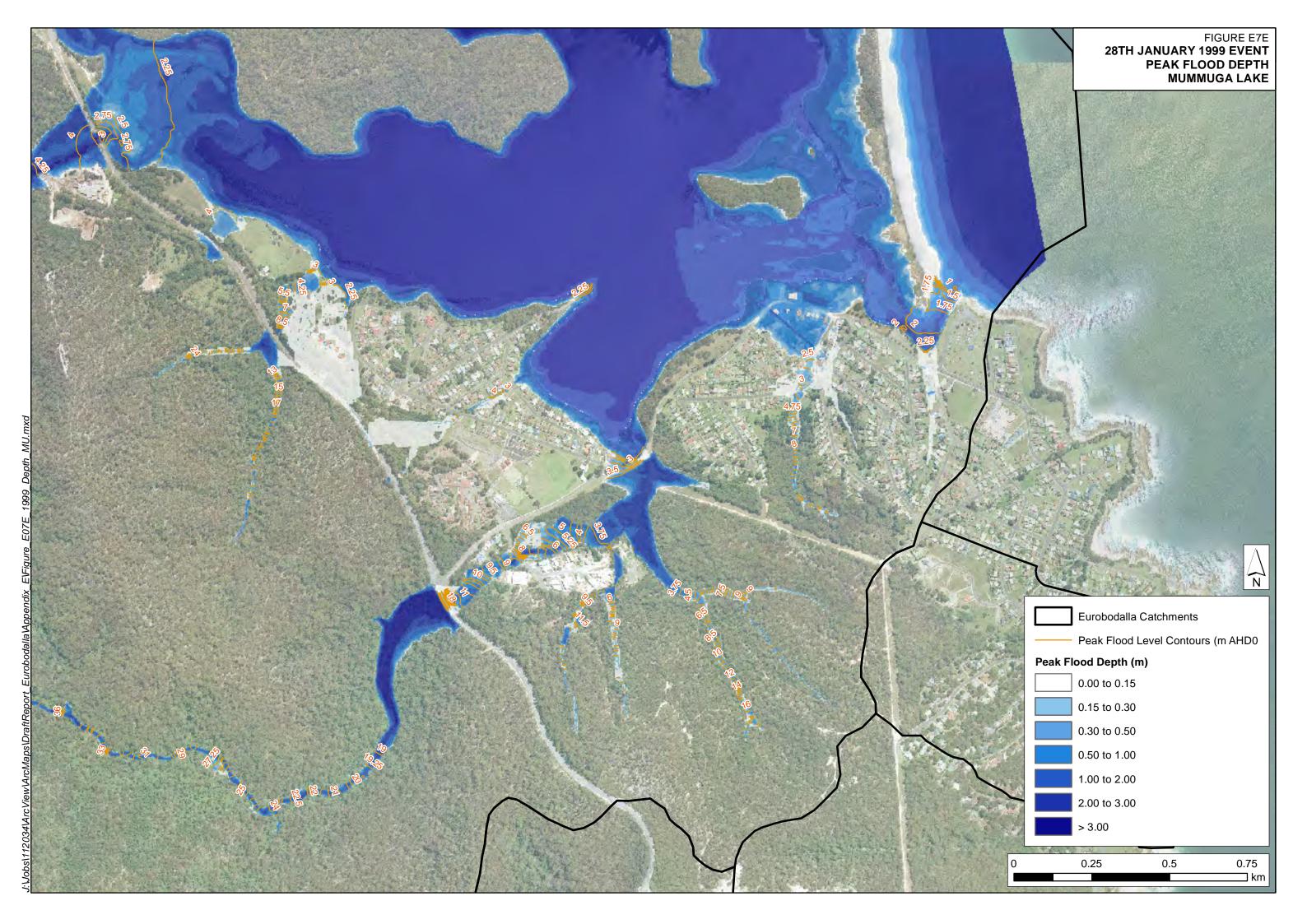


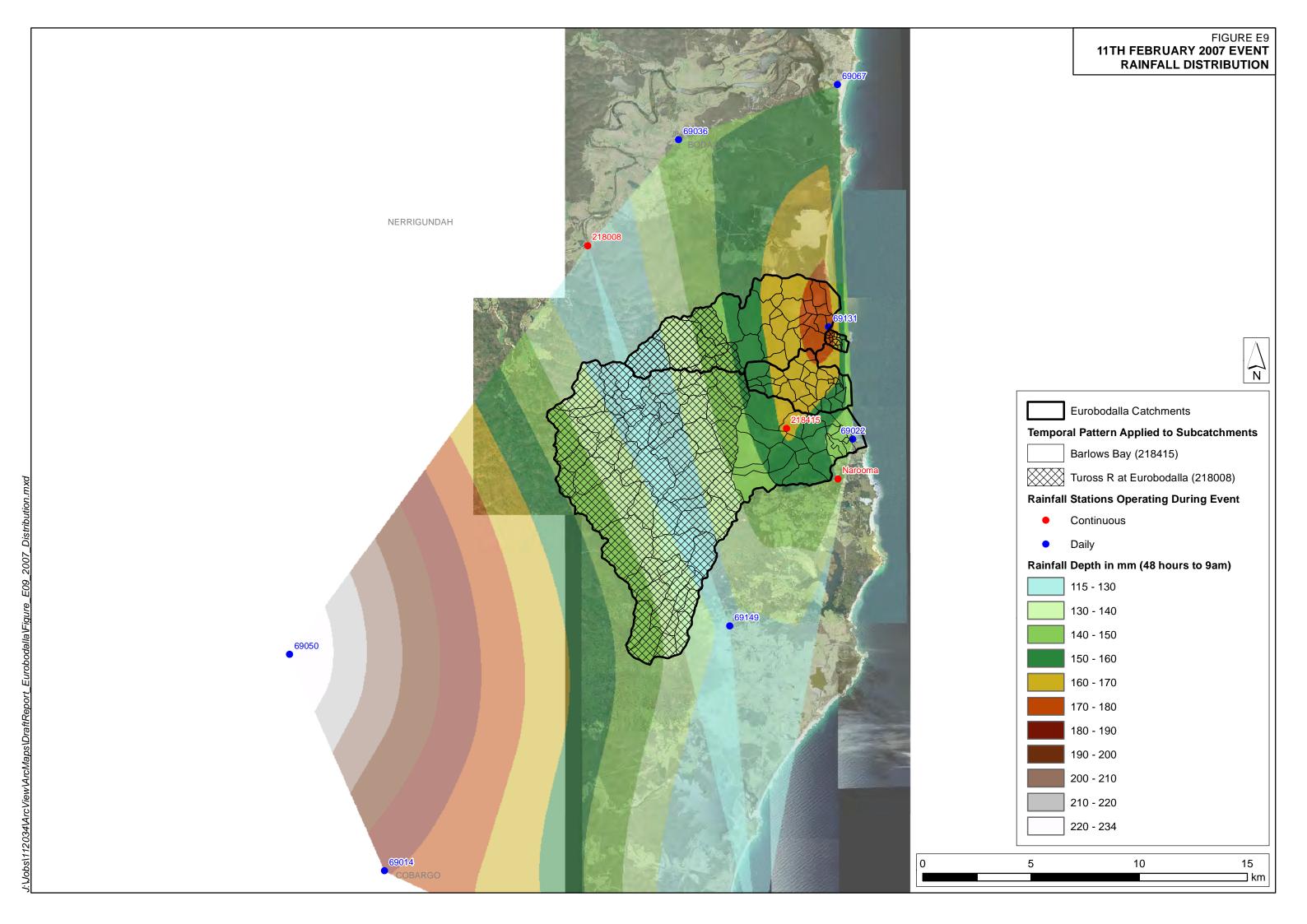




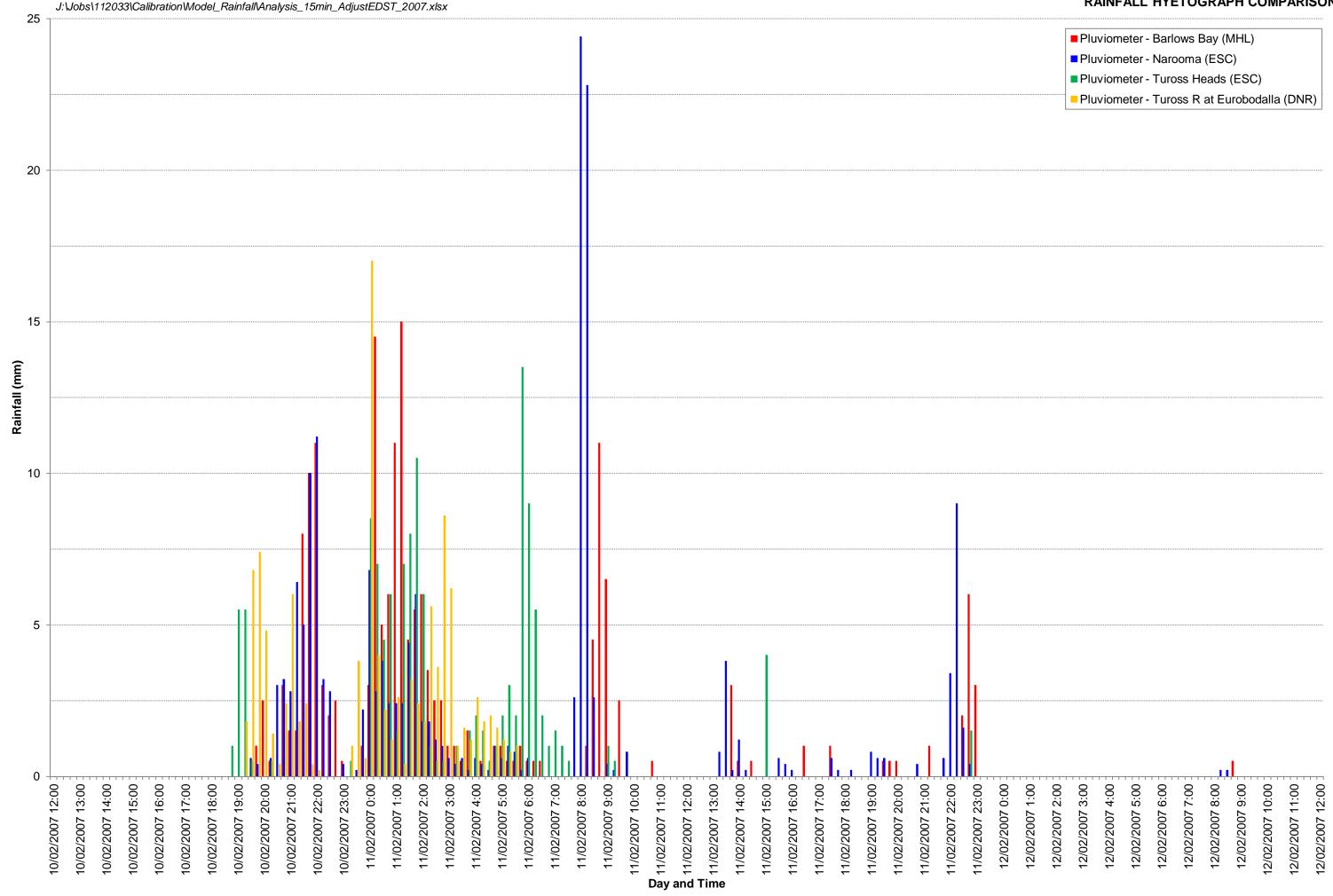




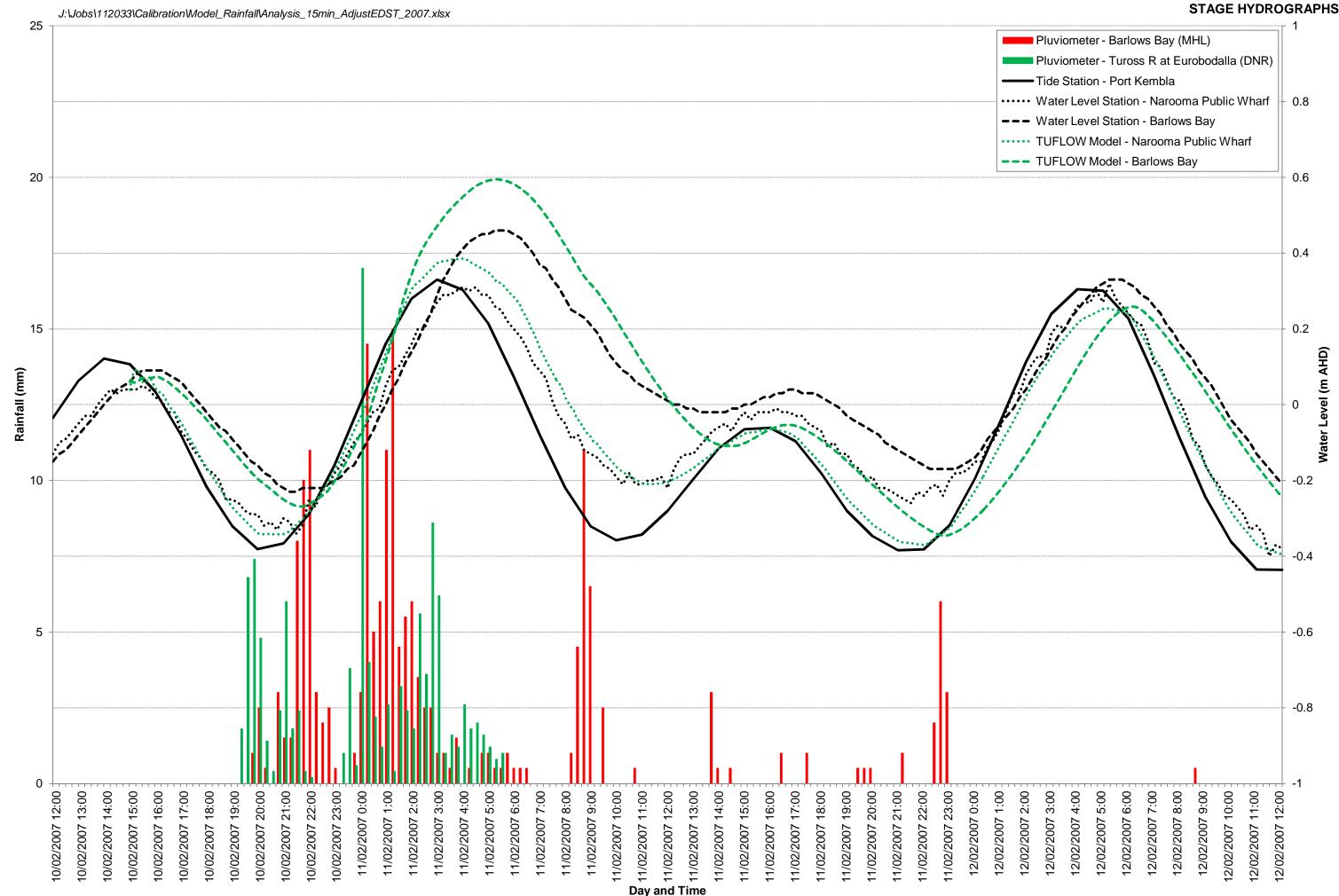


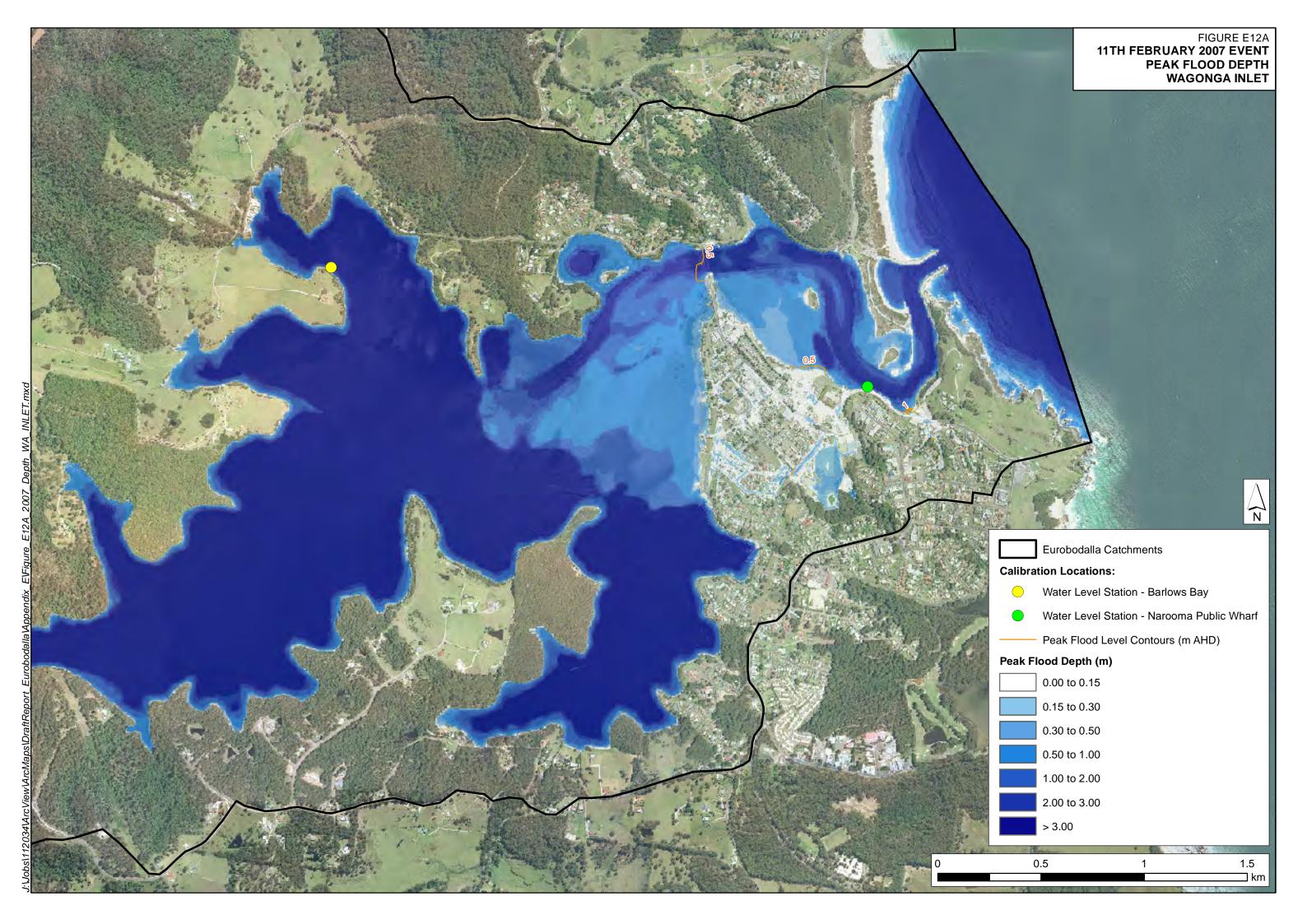


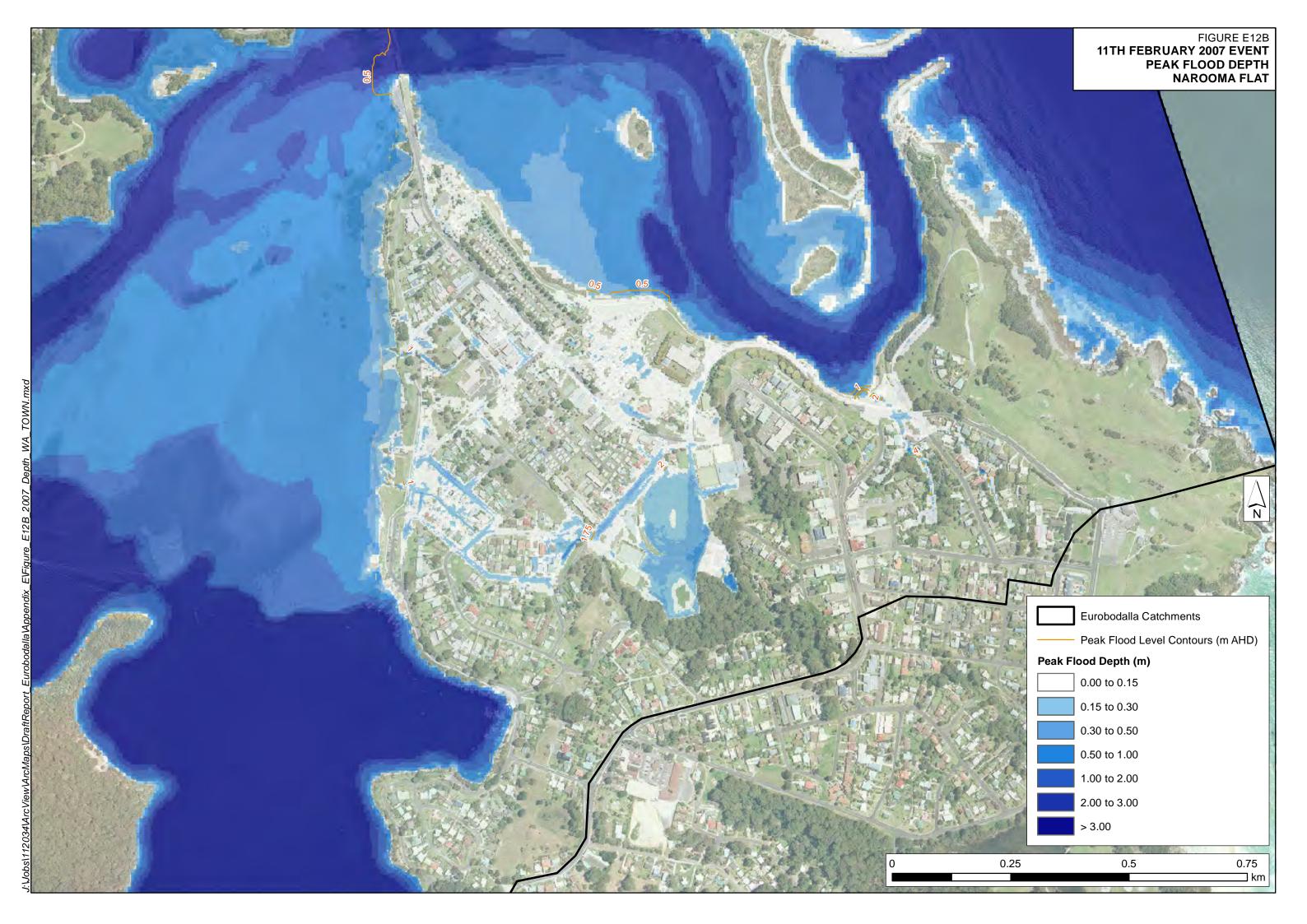


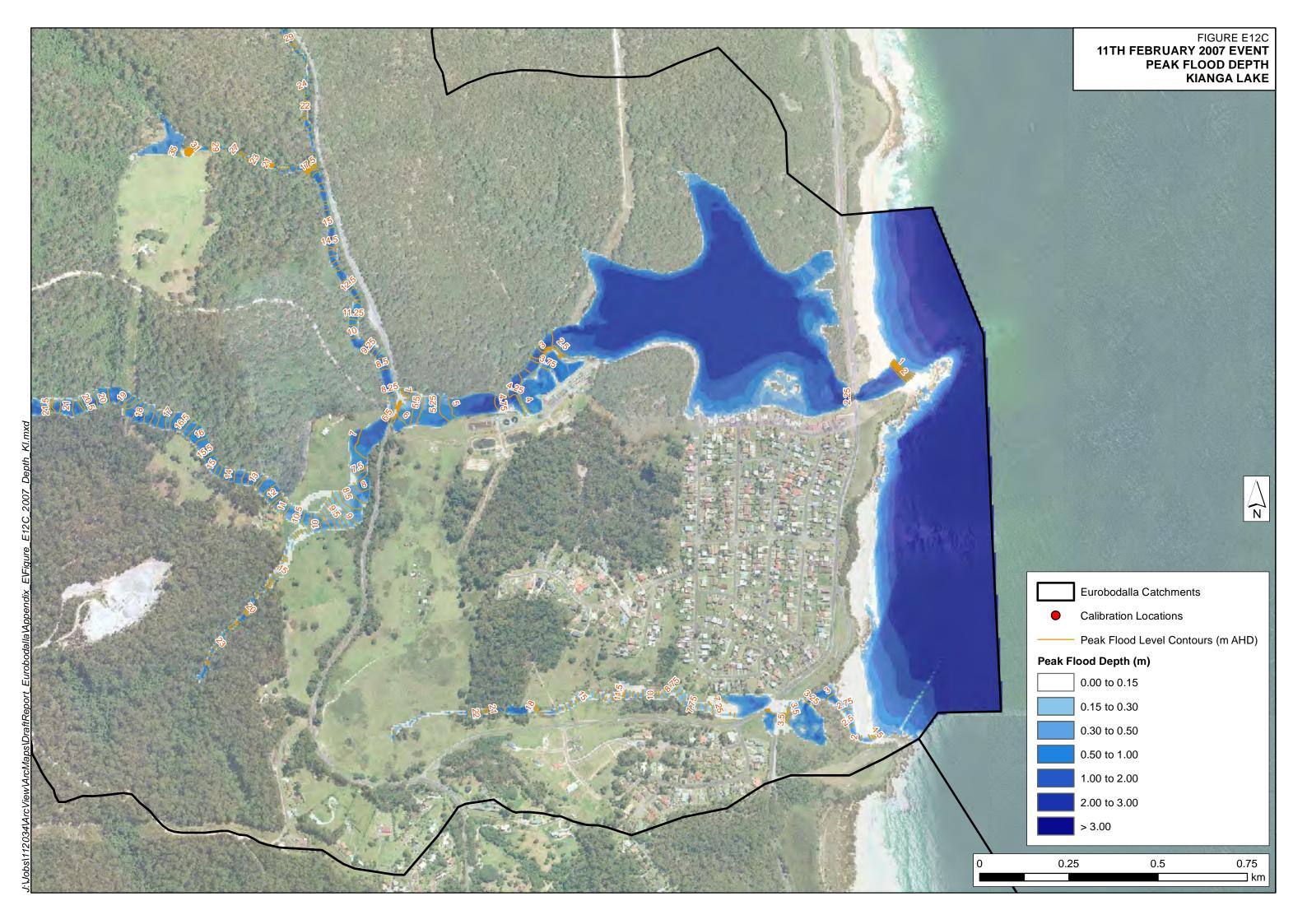


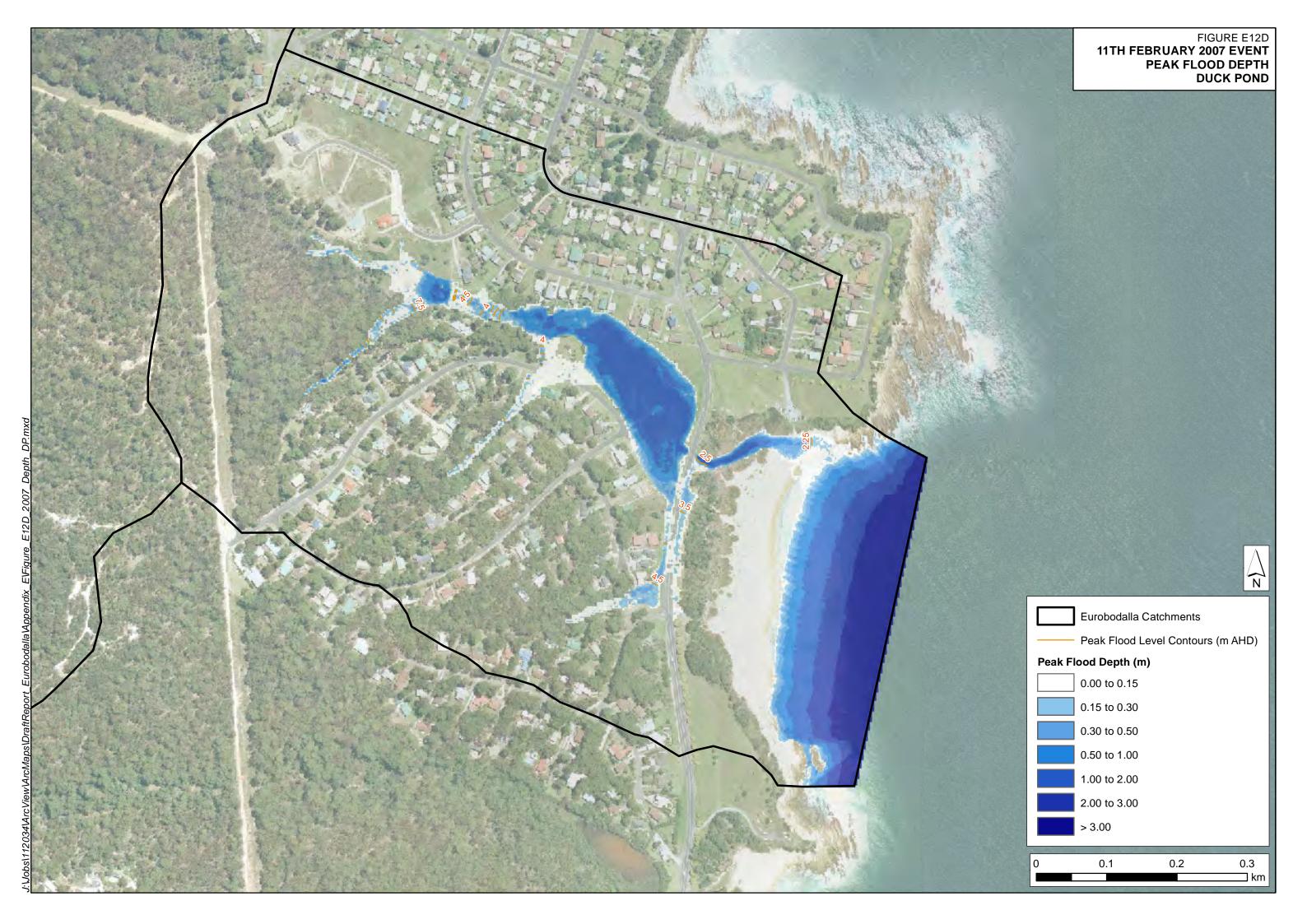
11TH FEBRUARY 2007 EVENT











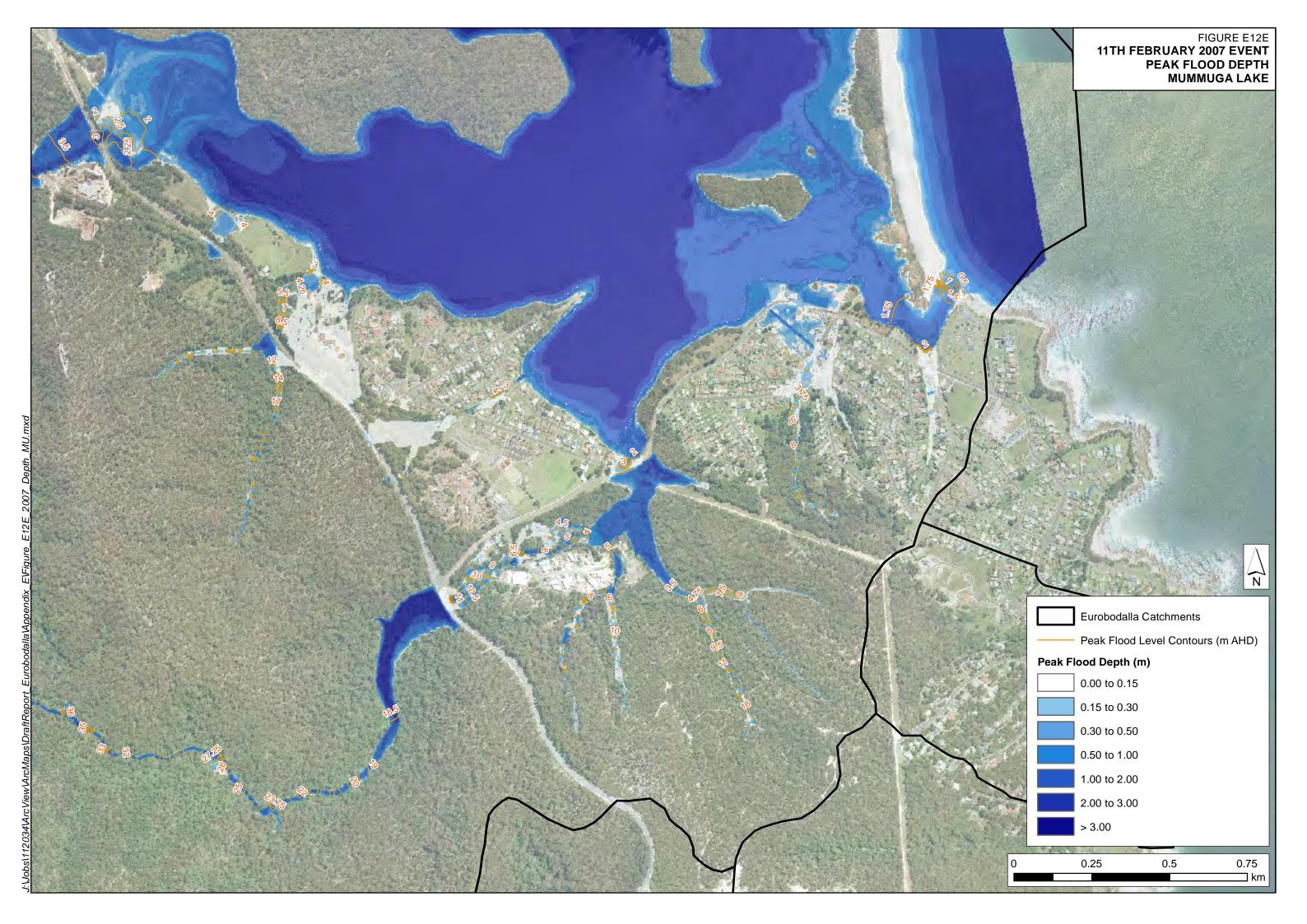
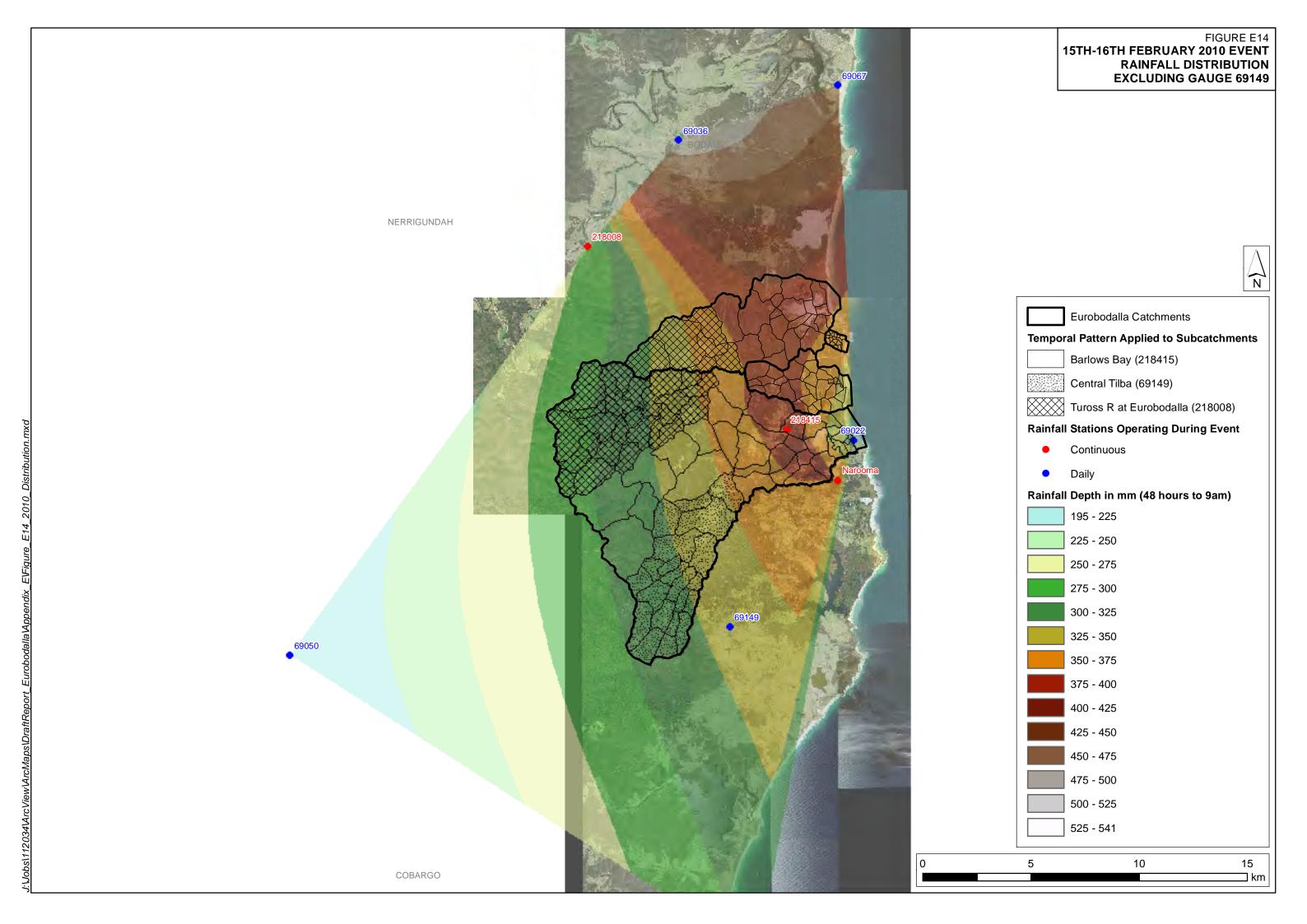
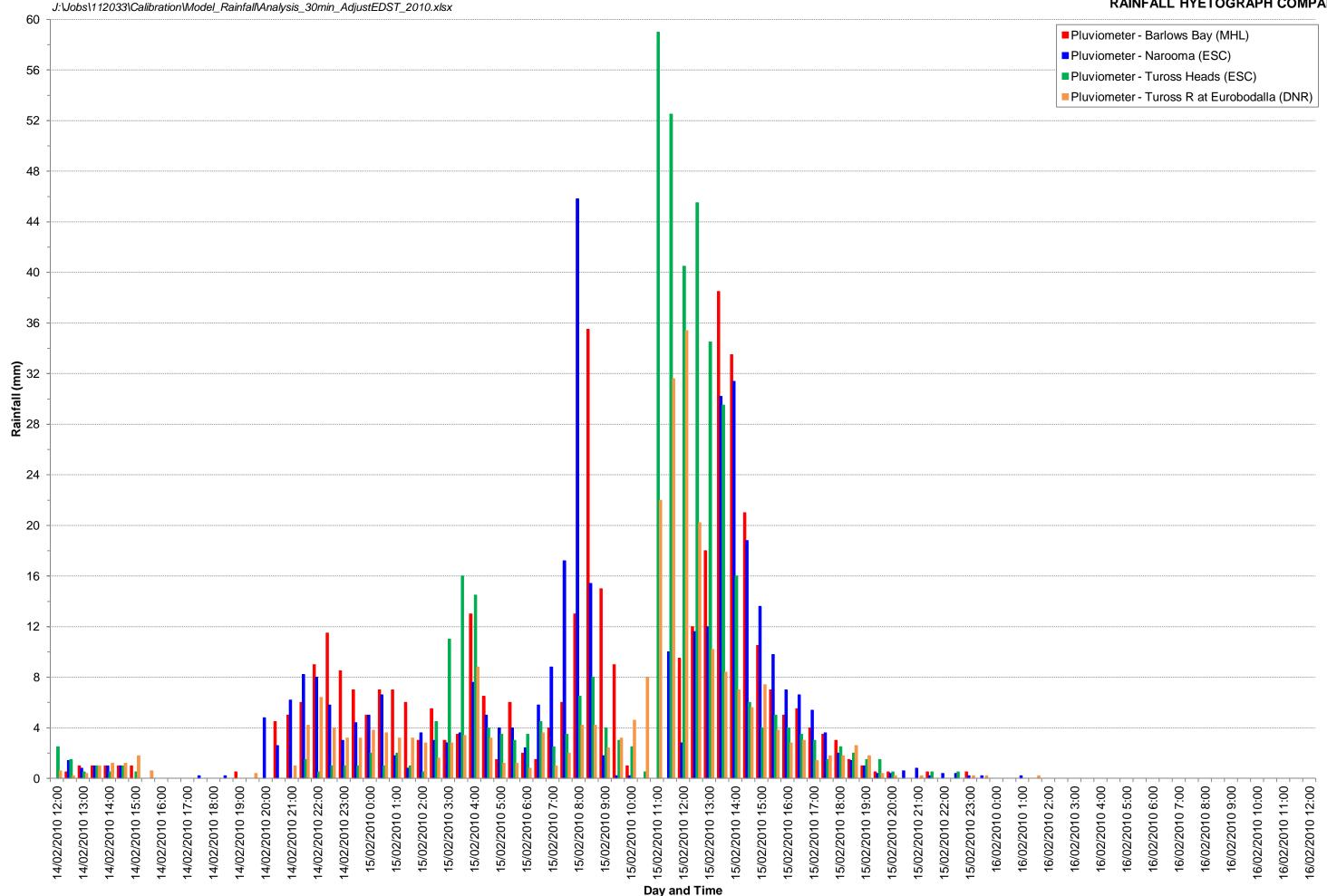
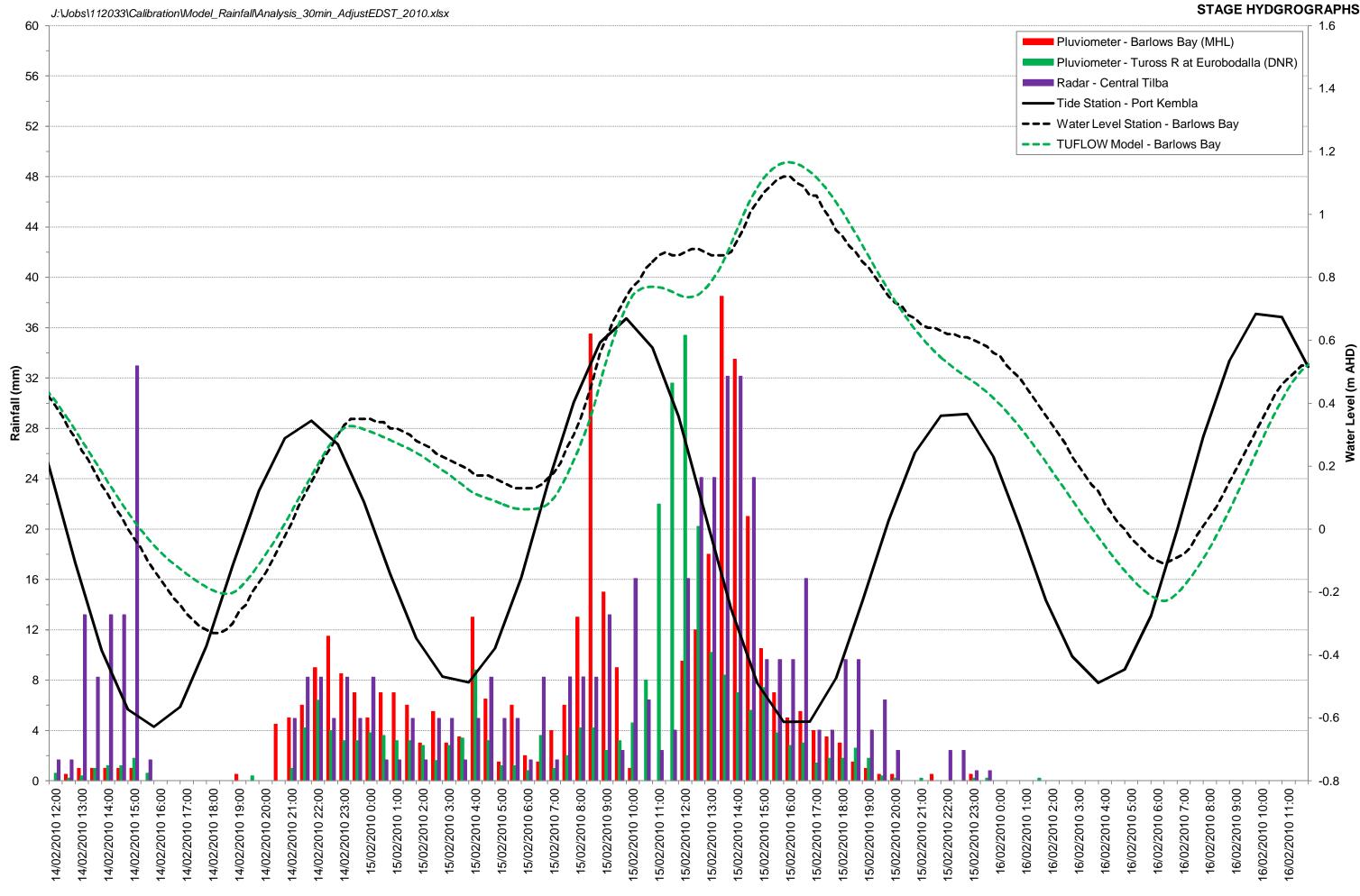


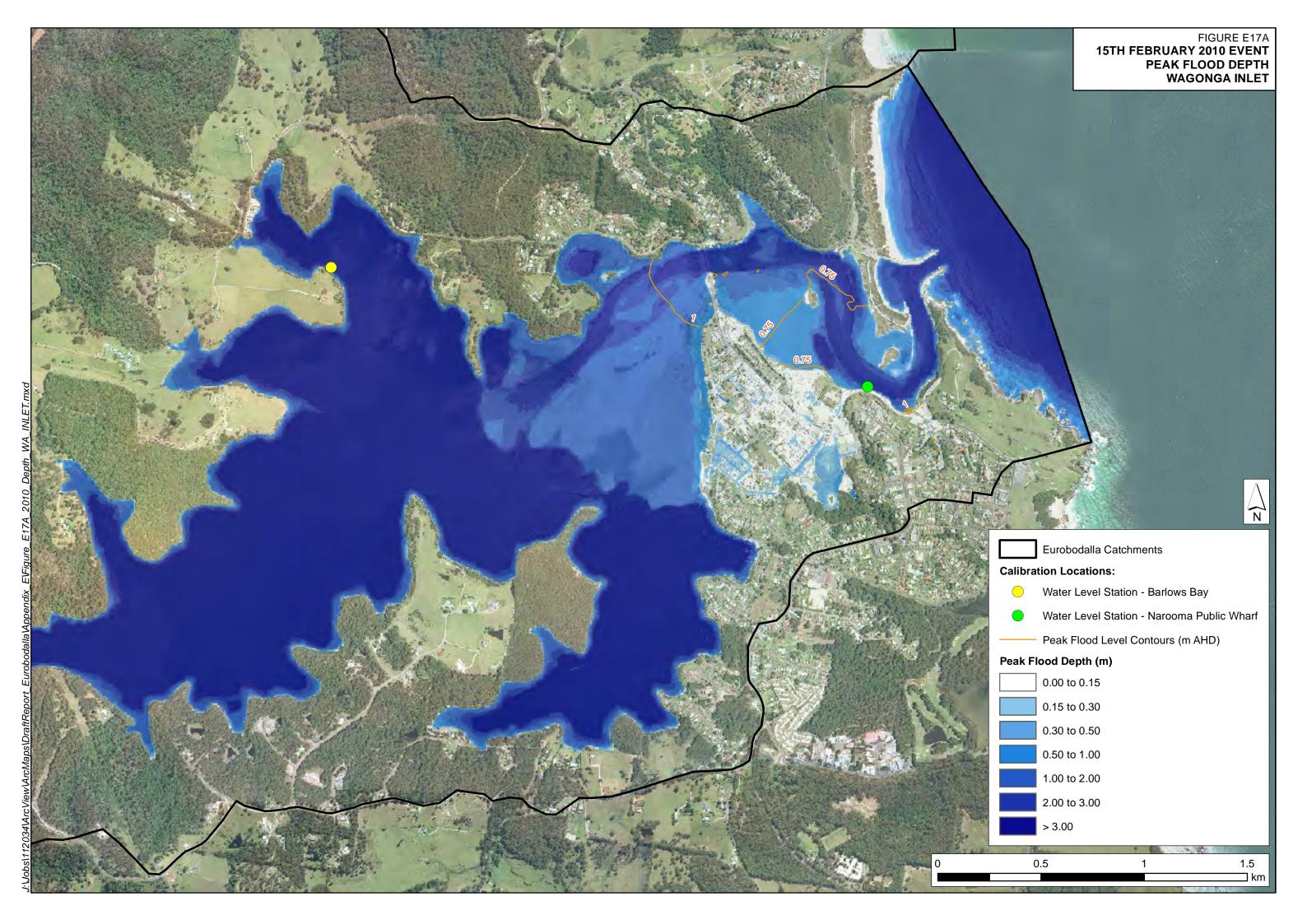
FIGURE E13
15TH FEBRUARY 2010 EVENT
IFD DATA

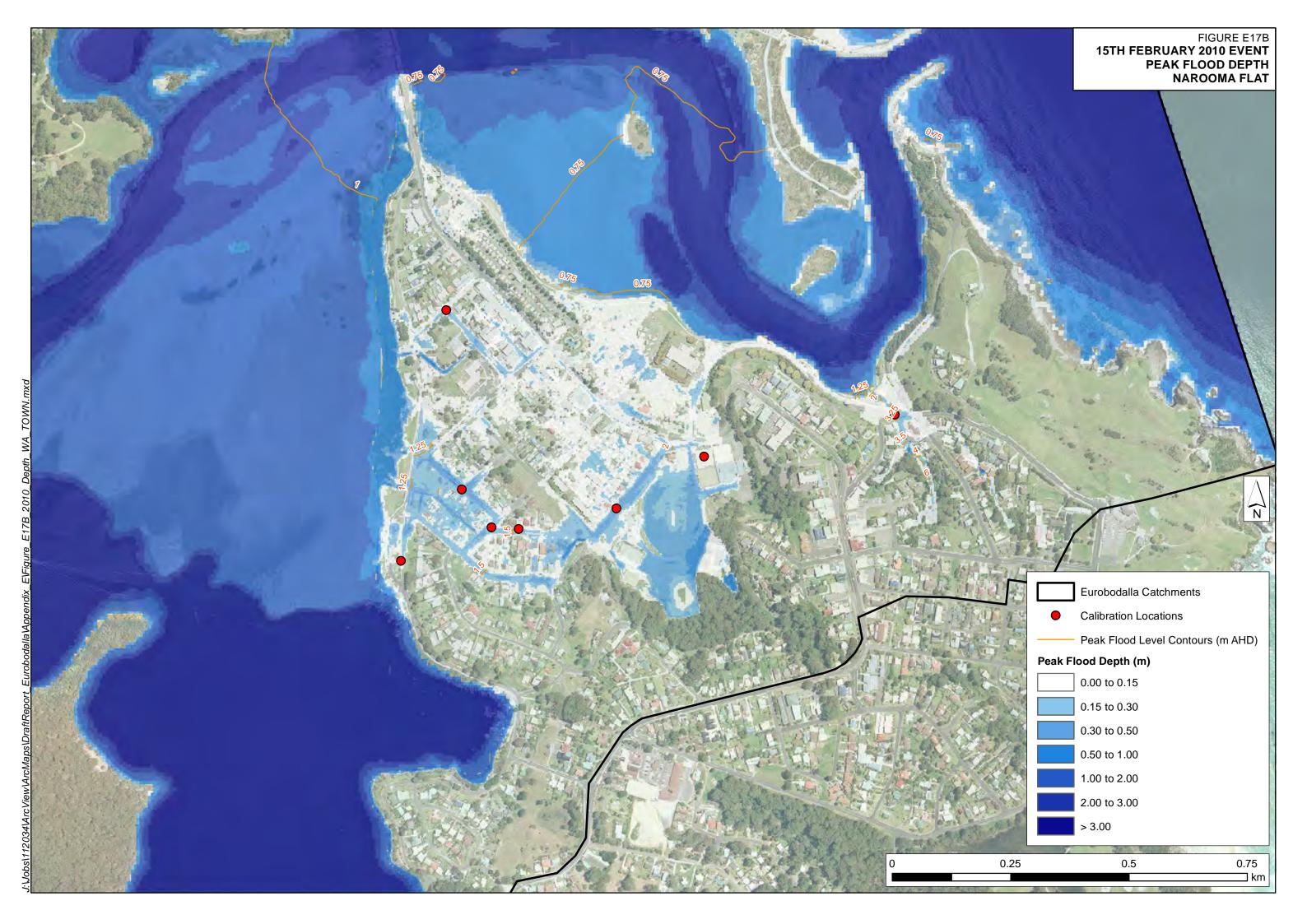


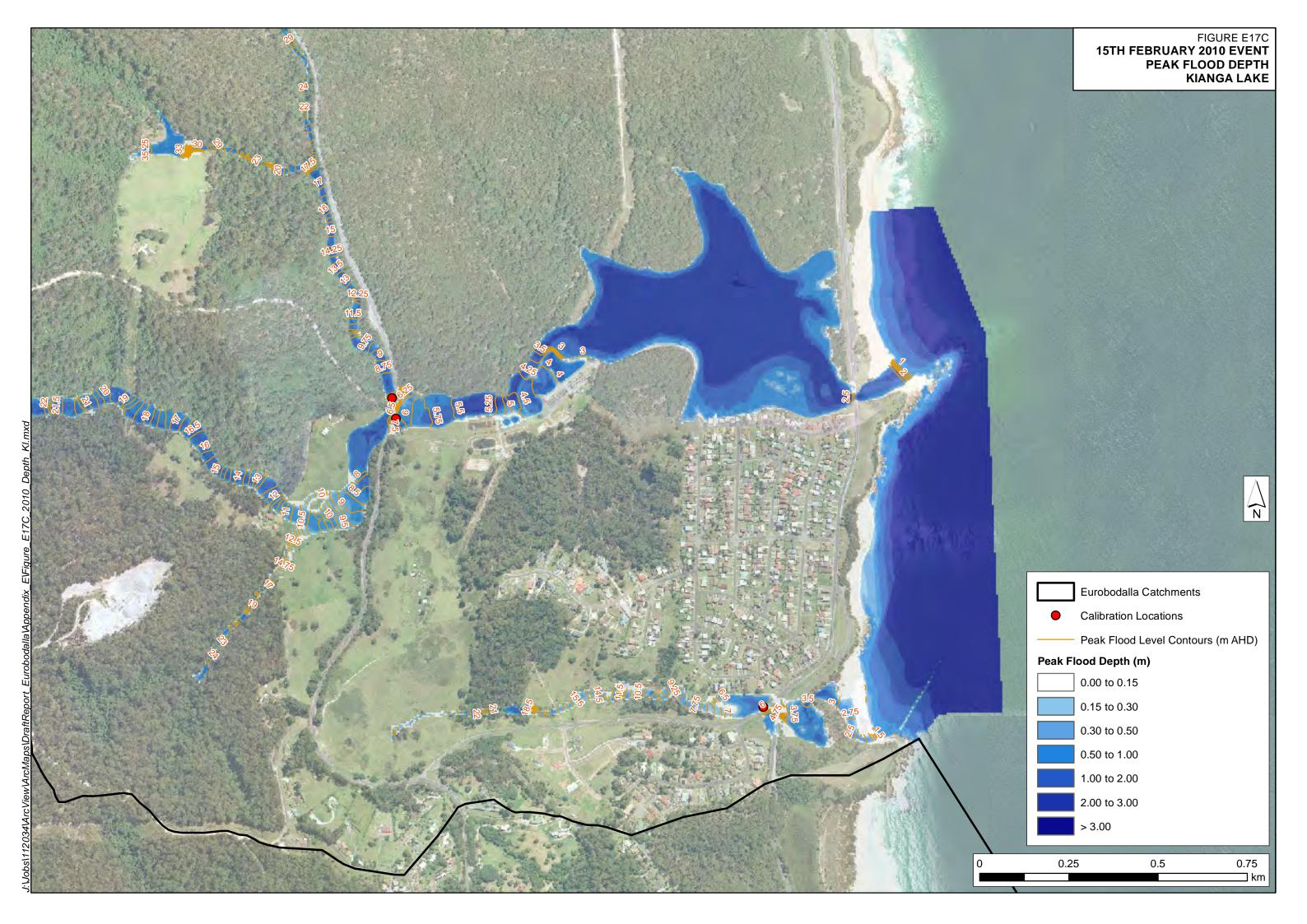


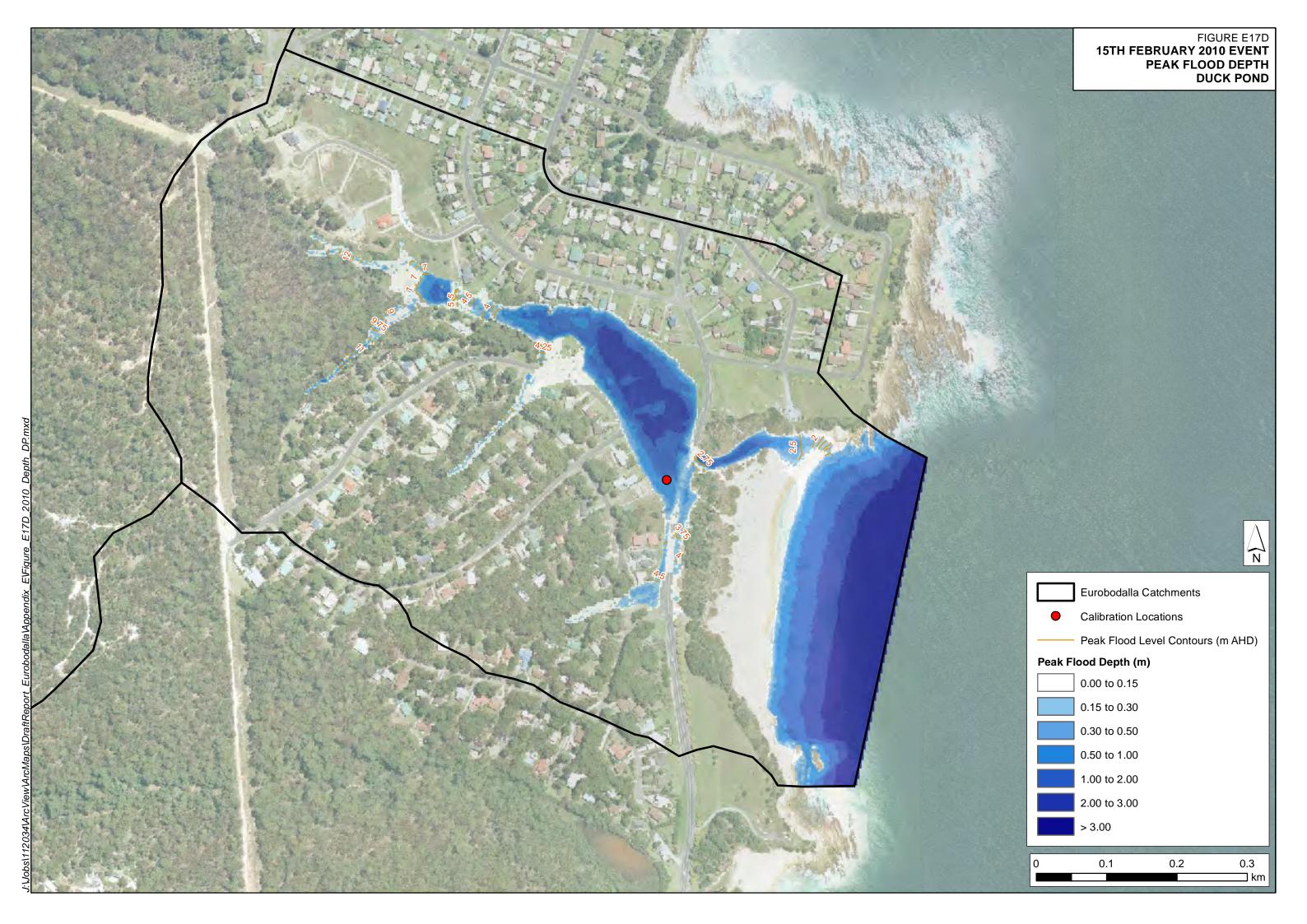


Day and Time









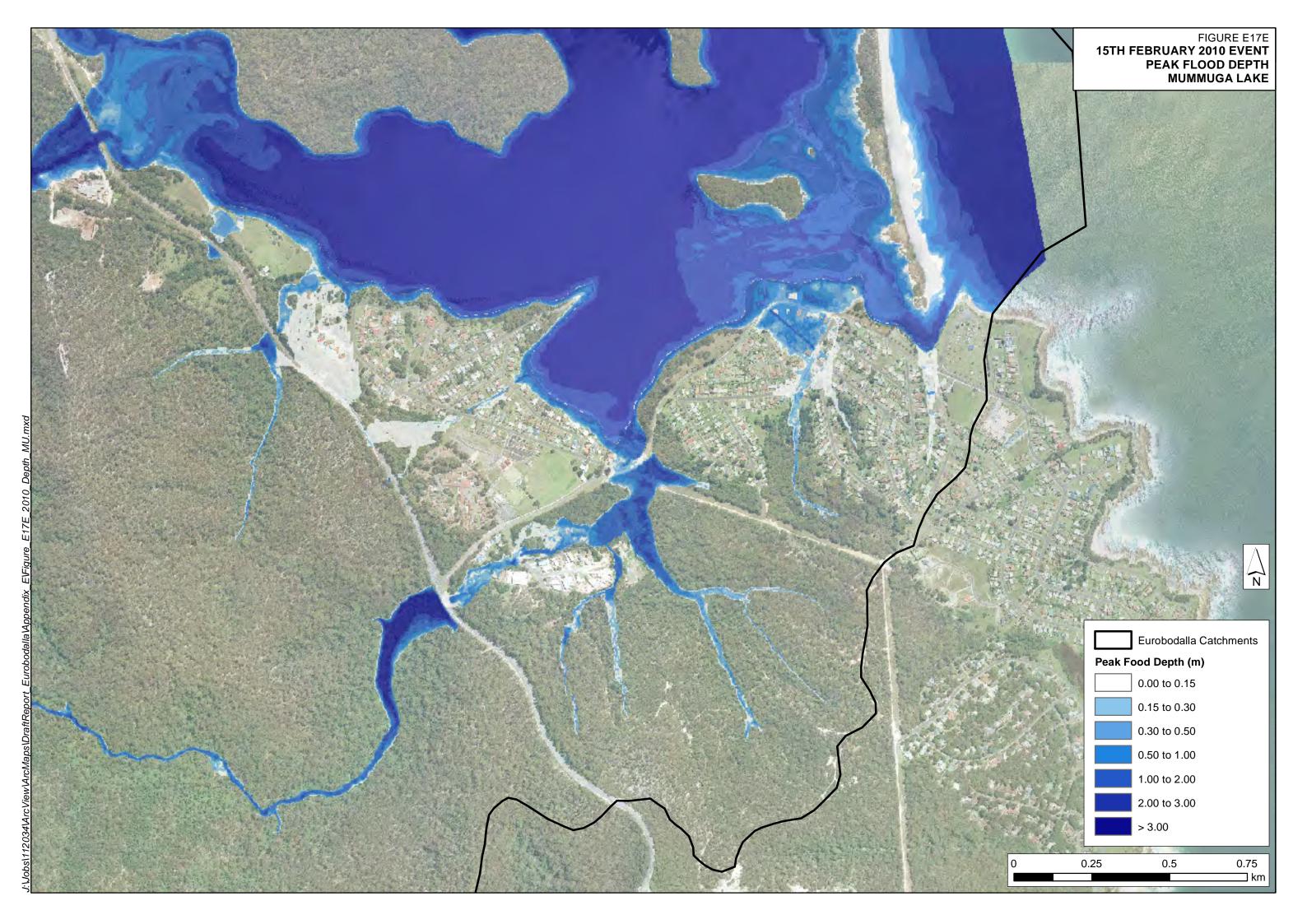
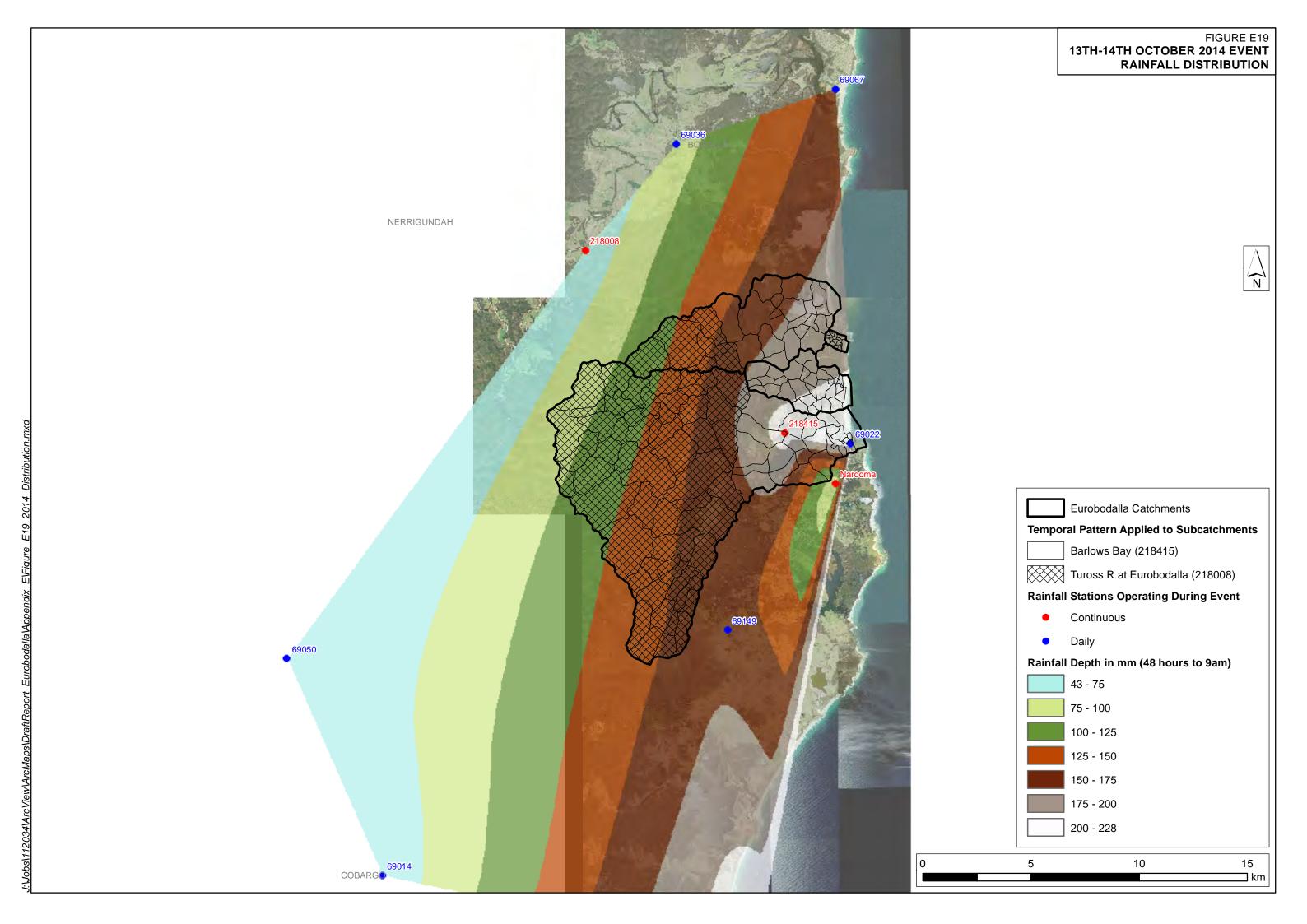
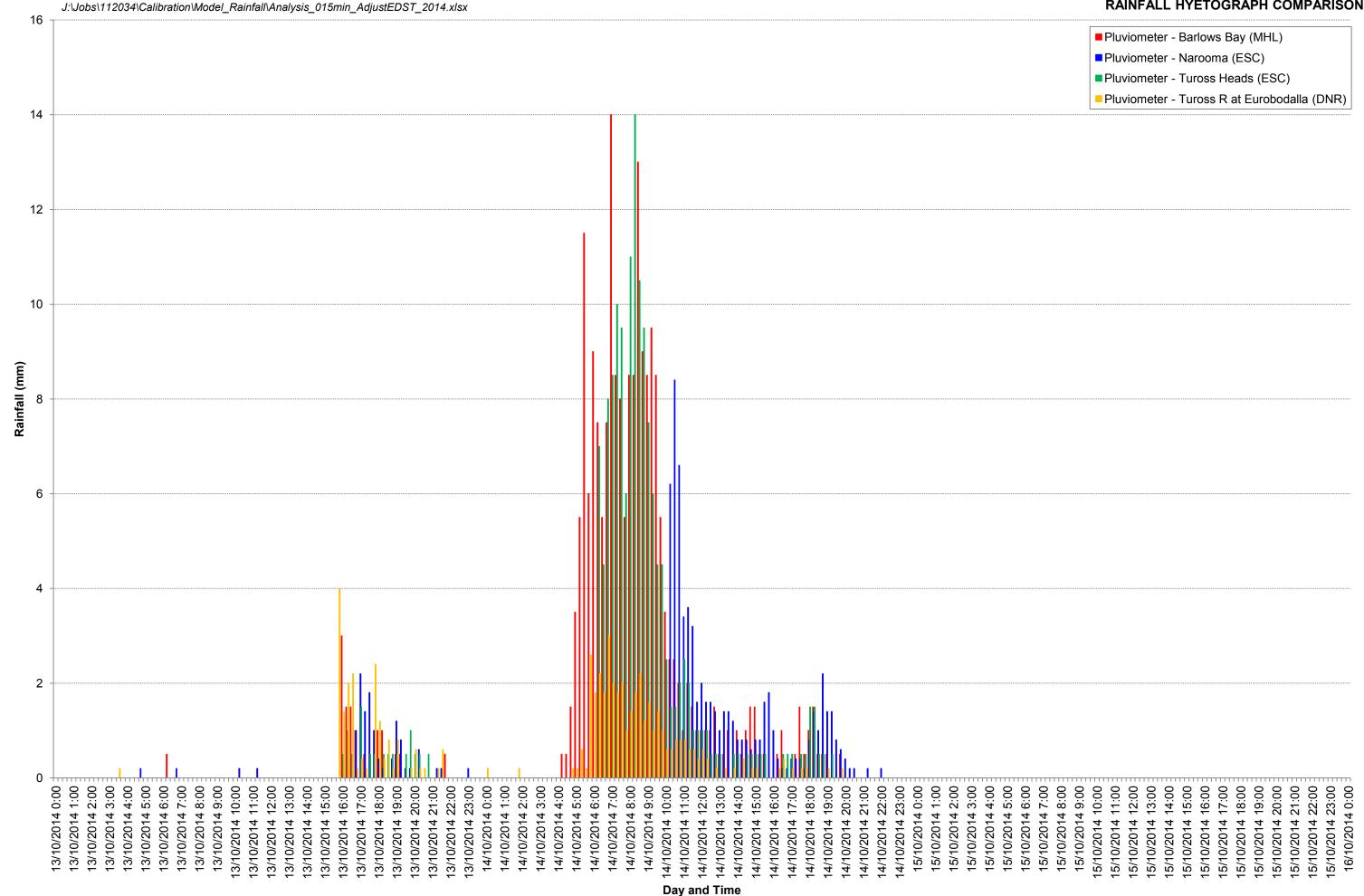
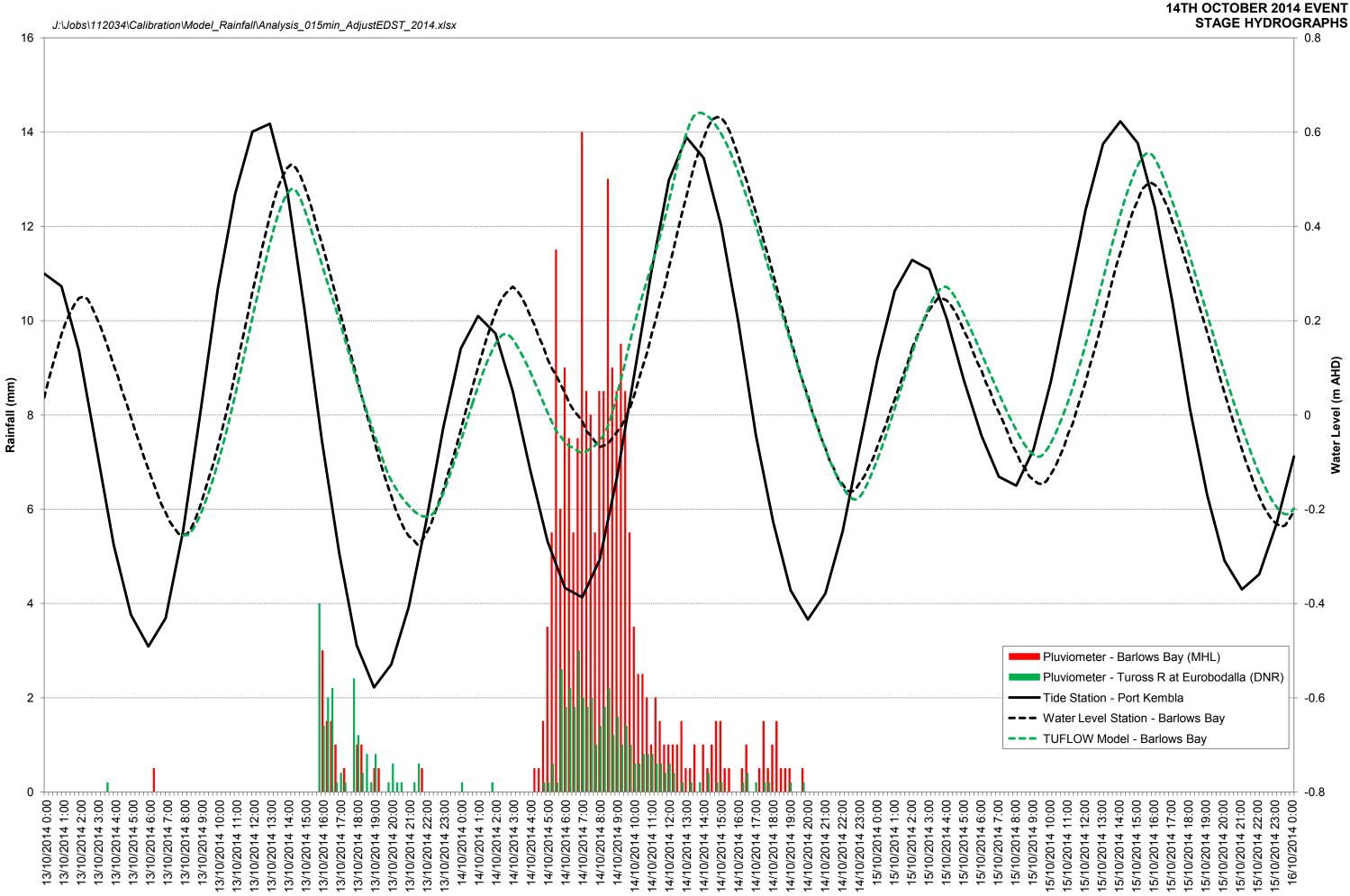


FIGURE E18
14TH OCTOBER 2014 EVENT
IFD DATA







Day and Time

